

West Lake Corridor Final Environmental Impact Statement/ Record of Decision and Section 4(f) Evaluation

Appendix E

Appendix E. Engineering Drawings (Part 8 of 10)



West Lake Corridor Final Environmental Impact Statement/ Record of Decision and Section 4(f) Evaluation

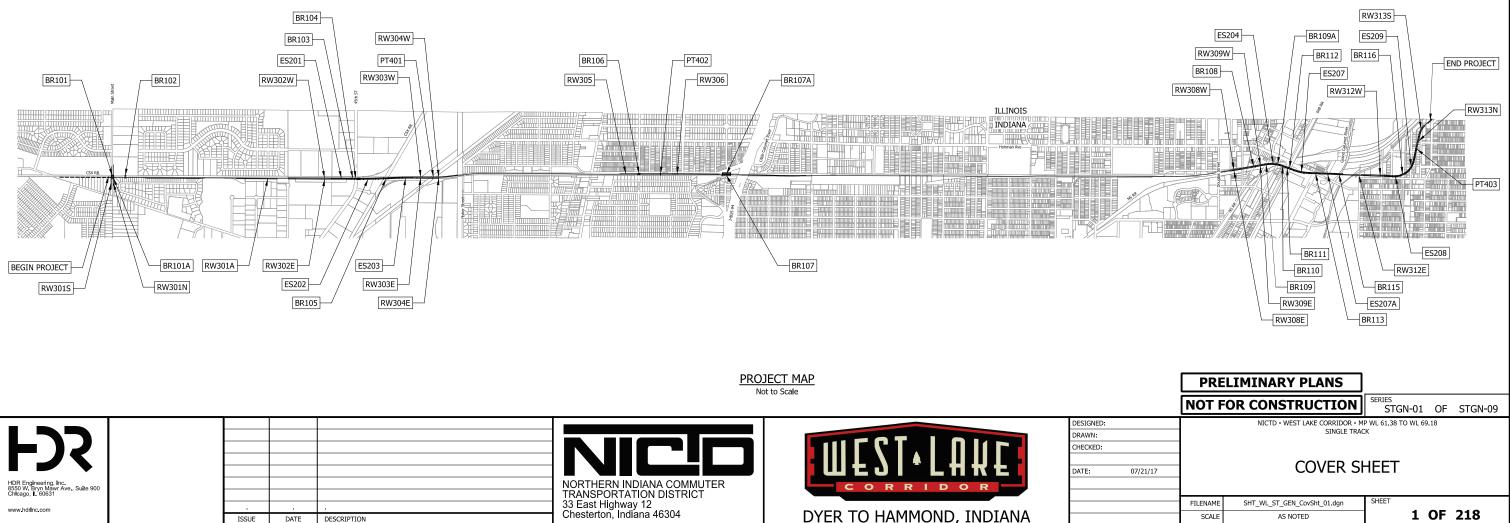
Appendix E

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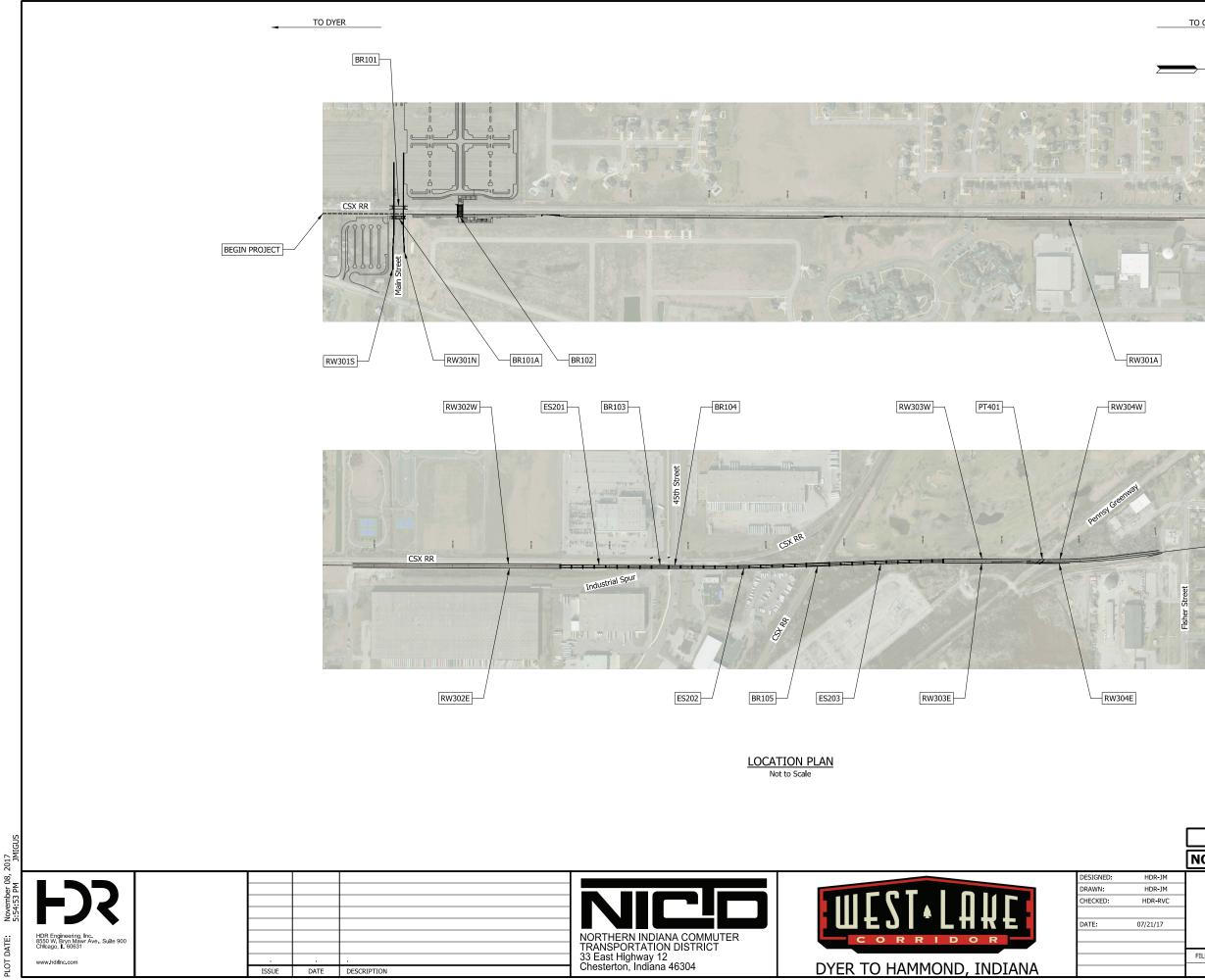
NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT

WEST LAKE CORRIDOR MILEPOST WL 61.38 TO MILEPOST WL 69.18 DYER TO HAMMOND, INDIANA

STRUCTURAL PLANS



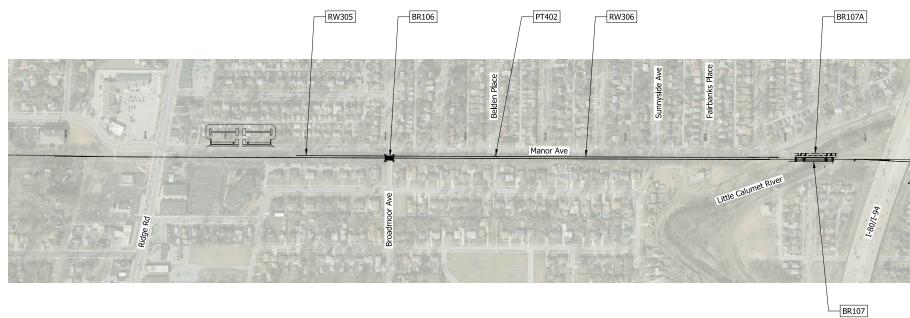
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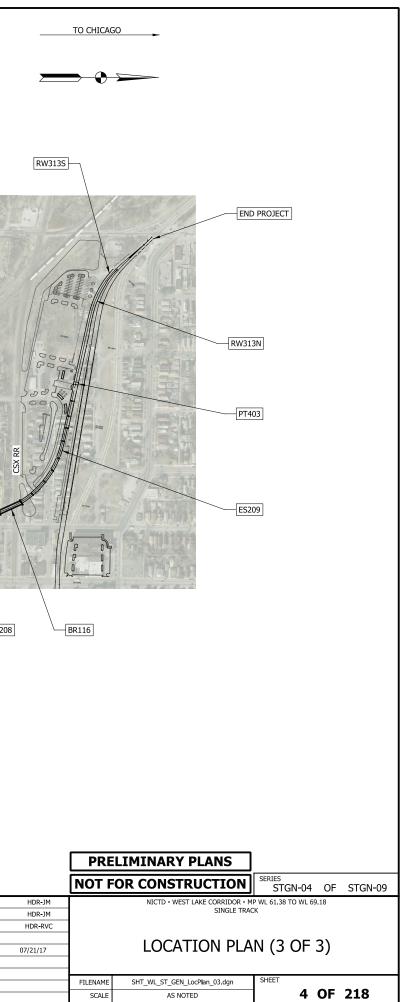




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STRUCTURAL SHEET INDEX

1	STGN-01		Cover Sheet
2	STGN-02 - STGN-04		Location Plan
5	STGN-05		Sheet Index
6 - 7	STGN-06 - STGN-07		General Notes
8 - 9	STGN-08 - STGN-09		Summary of Qu
10 - 13	BR101-01 - BR101-04	M.P. 61.37	Bridge 101 - CS
14 - 16	BR101A-01 - BR101A-03	N/A	Bridge 101A - Pe
17 - 20	RW301-01 - RW301-04	N/A	Retaining Walls
21 - 25	BR102-01 - BR102-05	M.P. 61.44	Bridge 102 - CS
26 - 31	RW301A-01 - RW301A-06	M.P. 62.08 - M.P. 62.32	Retaining Wall 3
32 - 37	RW302-01 - RW302-06	M.P. 62.38 - M.P. 62.63	Retaining Walls
38 - 43	ES201-01 - ES201-06	M.P. 62.68	Elevated Structu
44 - 47	BR103-01 - BR103-04	M.P. 62.75	Bridge 103 - NIG
48 - 51	BR104-01 - BR104-04	M.P. 62.77	Bridge 104 - NI
52 - 57	ES202-01 - ES202-06	M.P. 62.85	Elevated Structu
58 - 61	BR105-01 - BR105-04	M.P. 62.94	Bridge 105 - NIG
62 - 69	ES203-01 - ES203-08	M.P. 63.02	Elevated Structu
70 - 74	RW303-01 - RW303-05	M.P. 63.09 - M.P. 63.21	Retaining Walls
75	PT401-01	M.P. 63.21	Pedestrian Tunr
76 - 80	RW304-01 - RW304-05	M.P. 63.21 - M.P. 63.35	Retaining Walls
81 - 85	RW305-01 - RW305-05	M.P. 64.29 - M.P. 64.39	Retaining Walls
86 - 90	BR106-01 - BR106-05	M.P. 64.40	Bridge 106 - NIG
91 - 101	RW306-01 - RW306-11	M.P. 64.40 - M.P. 64.86	Retaining Wall 3
102	PT402-01	M.P. 64.52	Pedestrian Tunn
103 - 108	BR107-01 - BR107-06	M.P. 64.90	Bridge 107 - NIC
109 - 112	BR107A-01 - BR107A-04	N/A	Bridge 107A - P
113 - 118	RW308-01 - RW308-06	M.P. 67.78 - M.P. 67.93	Retaining Walls
119 - 122	BR108-01 - BR108-04	M.P. 67.94	Bridge 108 - NIC
123 - 126	RW309-01 - RW309-04	M.P. 67.95 - M.P. 68.00	Retaining Walls
127 - 131	BR109-01 - BR109-05	M.P. 68.01	Bridge 109 - NI
132 - 137	ES204-01 - ES204-06	M.P. 68.05	Elevated Structu
138 - 140	BR109A-01 - BR109A-03	M.P. 68.08	Bridge 109A - N
141 - 144	BR110-01 - BR110-04	M.P. 68.11	Bridge 110 - NIC
145 - 148	BR111-01 - BR111-04	M.P. 68.14	Bridge 111 - NIC
149 - 152	BR112-01 - BR112-04	M.P. 68.15	Bridge 112 - NI
153 - 160	ES207-01 - ES207-08	M.P. 68.22	Elevated Structu
161 - 166	BR113-01 - BR113-06	M.P. 68.30	Bridge 113 - NIC
167 - 175	ES207A-01 - ES207A-09	M.P. 68.37	Elevated Structu
176 - 180	BR115-01 - BR115-05	M.P. 68.44	Bridge 115 - NIG
181 - 186	RW312-01 - RW312-06	M.P. 68.46 - M.P. 68.71	Retaining Walls
187 - 193	ES208-01 - ES208-07	M.P. 68,75	Elevated Structu
194 - 198	BR116-01 - BR116-05	M.P. 68.81	Bridge 116 - NIG
199 - 211	ES209-01 - ES209-13	M.P. 68.90	Elevated Structu
212 - 217	RW313-01 - RW313-06	M.P. 68.97 M.P. 69.13	Retaining Walls
218	PT403-01	M.P. 68.98	Pedestrian Tunr

Sheet Index
General Notes
Summary of Quantities
Bridge 101 - CSX RR over Main Street
Bridge 101A - Pedestrian Bridge over Main Street
Retaining Walls 301S & 301N
Bridge 102 - CSX RR/NICTD over Pedestrian Underpass
Retaining Wall 301A
Retaining Walls 302W & 302E
Elevated Structure 201
Bridge 103 - NICTD over Industrial Spur
Bridge 104 - NICTD over 45th Street
Elevated Structure 202
Bridge 105 - NICTD over CSX RR
Elevated Structure 203
Retaining Walls 303W & 303E
Pedestrian Tunnel 401 at Pennsy Greenway
Retaining Walls 304W & 304E
Retaining Walls 305W & 305E
Bridge 106 - NICTD over Broadmoor Avenue
Retaining Wall 306
Pedestrian Tunnel 402 at Beldon Place
Bridge 107 - NICTD over Little Calumet River
Bridge 107A - Pedestrian Bridge over Little Calumet River
Retaining Walls 308W & 308E
Bridge 108 - NICTD over Fayette Street
Retaining Walls 309W & 309E
Bridge 109 - NICTD over Sibley Street
Elevated Structure 204
Bridge 109A - NICTD over State Street
Bridge 110 - NICTD over NS RR
Bridge 111 - NICTD over Willow Court
Bridge 112 - NICTD over IHB RR
Elevated Structure 207
Bridge 113 - NICTD over Hohman Avenue
Elevated Structure 207A
Bridge 115 - NICTD over Grand Calumet River
Retaining Walls 312W & 312E
Elevated Structure 208
Bridge 116 - NICTD over CSX RR
Elevated Structure 209
Retaining Walls 313W & 313E
Pedestrian Tunnel 403 at Gateway Station

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ISSUE DATE DESCRIPTION

GENERAL NOTES:

DESIGN

STRUCTURES ARE TO BE DESIGNED PER CURRENT EDITION OF THE AREMA MANUAL FOR RAILWAY ENGINEERING (AREMA MANUAL).

DESIGN CRITERIA:

DEAD LOAD:	SELF-WEIGHT OF STRUCTURE, WEIGHT OF RAIL AND FASTENINGS, BALLAST, DECK, WATERPROOFING, WALKWAY, AND OTHER MISCELLANEOUS FIXTURES
LIVE LOAD:	DESIGN OF ALL STRUCTURES SHALL SATISFY COOPER E50 LIVE LOAD, ALONG WITH PROVISIONS FOR PERIODIC SERVICE OF 263K WORK TRAINS WITH GP38 4-AXLE LOCOMOTIVES. COOPER E80 LIVE LOAD AND ALTERNATE LIVE LOAD AS APPROPRIATE SHALL BE SATISFIED ON ALL CSX BRIDGES
IMPACT:	IMPACT LOAD SHALL BE PER AREMA CHAPTERS 8 AND 15 AS APPLICABLE
FATIGUE:	PER AREMA 15-1.3.13 FATIGUE
WIND LOAD:	PER AREMA 8-2.2.3, 15-1.3.7 AND 15-1.3.8 AS REQUIRED

EXISTING CONDITIONS:

ALL DIMENSIONS OF EXISTING CONDITIONS TO BE VERIFIED BY THE CONTRACTOR

DESIGN	STRESSES:

STRUCTURAL STEEL (ASTM A709, GR. 50 AND 50W)	Fy = 50,000 PSI
CAST-IN-PLACE CONCRETE (INDOT CLASS C)	F'c = 4000 PSI AT 28 DAYS
REINFORCEMENT STEEL (ASTM A615)	Fy = 60 KSI

CONTROL OF ALL WORK

ALL WORK INVOLVED IN THE CONSTRUCTION OF THE RAILWAY STRUCTURE SHALL BE PERFORMED SATISFACTORY TO THE ENGINEER AND NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT (NICTD). ALL METHODS OF HANDLING WORK AFFECTING THE SAFETY OF RAIL OPERATIONS MUST BE APPROVED BY THE RAILWAY ENGINEER BEFORE PROCEEDING WITH THAT PORTION OF THE WORK. RAIL TRAFFIC SHALL AT ALL TIMES BE MAINTAINED AND PROTECTED. THE CONTRACTOR SHALL NOT AT ANY TIME DELAY OR INTERFERE WITH RAIL OPERATIONS.

CONSTRUCTION REQUIREMENTS:

ALL WORK SHALL BE IN ACCORDANCE WITH CURRENT AREMA "MANUAL FOR RAILWAY ENGINEERING" AND THE SPECIFICATIONS FOR THIS CONTRACT. THE CONTRACTOR SHALL NOT INTERFERE WITH OR PERFORM ANY CONSTRUCTION ON OR NEAR OPERATING TRACKS WITHOUT THE RAILROAD'S PERMISSION. WHEN THE CONTRACTOR IS WORKING NEAR ANY TRACK, HE WILL BE REQUIRED TO HAVE A FLAGMAN FROM THE RAILROAD ON DUTY. CONTRACTOR SHALL NOT SCALE DIMENSIONS FROM THE CONTRACT PLANS FOR CONSTRUCTION PURPOSES. SCALES ARE SHOWN FOR INFORMATION ONLY. NO CONSTRUCTION JOINTS, EXCEPT THOSE SHOWN ON THE PLANS, WILL BE ALLOWED UNLESS APPROVED BY THE ENGINEER.

STRUCTURAL STEEL NOTES:

GENERAL:

- 1. PRIOR TO FABRICATION, CONTRACTOR/FABRICATOR SHALL SUBMIT THE FOLLOWING FOR APPROVAL BY THE ENGINEER:
 - SHOP DRAWINGS INDICATING MATERIALS, SIZES, CONNECTIONS, ANCHORS, PAINTING (AS APPLICABLE), WELD PROCEDURES, AND CERTIFICATIONS.
 - PRODUCT DATA INCLUDING MANUFACTURER'S CATALOG SHEETS ON PRE-MANUFACTURED ITEMS.
- 2. DELIVERY, STORAGE, AND HANDLING:
 - A. TAG MISCELLANEOUS STEEL, FASTENERS, AND ACCESSORIES OR OTHERWISE MARK FOR EASE OF IDENTIFICATION AT PROJECT SITE.
 - CONTRACTOR IS RESPONSIBLE FOR SAFELY TRANSPORTING, STORING, AND HANDLING ALL MATERIALS. ALL MATERIALS SHALL BE PROTECTED FROM DAMAGE AND THEFT DURING ALL PHASES OF CONSTRUCTION.
- 3 STRUCTURAL STEEL

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- STEEL FOR BRIDGE SUPERSTRUCTURE AND STEEL H-PILES, WITH THE EXCEPTION OF DECK PLATES, SHALL CONFORM TO THE REQUIREMENTS OF ASTM A709 GRADE 50W T2 UNLESS NOTED OTHERWISE. DECK PLATES SHALL CONFORM TO ASTM A709 GRADE 50 T2 FRACTURE CRITICAL MEMBERS AS DESCRIBED IN AREMA CHAPTER 15 PART 1.14 AND AS NOTED ON THE PLANS SHALL CONFORM TO ASTM A709 GRADE 50W F2.
- B. FABRICATE DETAILS AND CONNECTION ASSEMBLIES IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS, WITH PROJECTING CORNERS CLIPPED AND FILLER PIECES WELDED FLUSH.
- ALL MATERIAL SHALL BE STRAIGHT AND FREE FROM SHARP KINKS OR BENDS. ANY STEEL MATERIAL EXHIBITING SUCH DEFICIENCIES SHALL BE CAUSE FOR THE REJECTION OF THE MATERIAL. STRAIGHTENING OF THE MATERIAL SHALL NOT BE ACCEPTABLE.
- FIT WORK TOGETHER IN FABRICATION SHOP AND DELIVER COMPLETE OR IN PARTS, READY TO BE SET IN PLACE OR ASSEMBLED IN FIELD AS COORDINATED D. WITH THE CONTRACTOR.
- STEEL SUPERSTRUCTURE IS NOT INTENDED TO BE PAINTED EXCEPT THE ENDS OF BEAMS OR GIRDERS SHALL BE PAINTED TO PREVENT STAINING OF THE CONCRETE SUBSTRUCTURE. ENDS OF BEAMS OR GIRDERS, UP TO FIVE FEET INTO THE SPAN SHALL BE PAINTED TO MATCH/BLEND INTO THE NATURAL COLOR OF THE STEEL. COLOR SAMPLE IS TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

STRUCTURAL STEEL NOTES (CONTINUED):

GALVANIZING FOR HANDRAIL AND WALKWAY:

- A. GALVANIZE AFTER FABRICATION.
- GALVANIZE BY HOT-DIP PROCESS CONFORMING WITH ASTM A123 AND AMERICAN В. GALVANIZERS ASSOCIATION RECOMMENDATIONS.
- C. GALVANIZE IN PLANT HAVING FACILITIES TO PRODUCE QUALITY COATINGS AND CAPACITY FOR VOLUME OF WORK.
- D. SHIP AND HANDLE MATERIAL TO AVOID DAMAGE TO ZINC COATING.
- REPAIR GALVANIZED SURFACES DAMAGED DURING SHIPPING OR CONSTRUCTION Ε. OPERATIONS. REPAIR SURFACES ACCORDING TO ASTM A780 WITH ZINC-RICH PAINT. REPAIR PRODUCT IS TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL
- PLUMB AND TRUE VERTICAL AND HORIZONTAL MEMBERS TO TOLERANCE OF +/- 1/8" IN 10 FT. 5.
- PROVIDE SHIMS, BLOCKS, WEDGES, AND OTHER ITEMS AS NECESSARY TO COMPLETE 6 INSTALLATION AT NO ADDITIONAL COST TO THE PROJECT

WELDING:

ALL WELDING SHALL BE IN ACCORDANCE WITH CURRENT AREMA MANUAL AND THE PROJECT SPECIFICATIONS. WELDING IS TO BE IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (A.W.S.) STRUCTURAL WELDING CODE D1.5, CURRENT ISSUE. ALL WELDS TO BE CONTINUOUS UNLESS OTHERWISE SHOWN. ALL WELDERS TO BE QUALIFIED IN ACCORDANCE WITH A.W.S. STRUCTURAL WELDING CODE OR OTHER REGULATIONS MEETING WITH THE APPROVAL OF THE ENGINEER, FIELD WELDS ARE TO BE MADE WITH E7018 LOW HYDROGEN ELECTRODES WITH ON-SITE PROTECTION AND USE OF ELECTRODE HEATING UNITS PER CURRENT A.W.S. SPECIFICATIONS.

- CONFORM TO A.W.S. SPECIFICATIONS FOR ARC AND GAS WELDING IN BRIDGE CONSTRUCTION AND TO AISC SPECIFICATIONS. SURFACES TO BE WELDED SHALL BE FREE FROM LOOSE SCALE, RUST, GREASE, PAINT, AND OTHER FOREIGN MATERIAL, EXCEPT MILL SCALE WHICH WILL WITHSTAND VIGOROUS WIRE BRUSHING MAY REMAIN. NO WELDING SHALL BE DONE WHEN BASE METAL TEMPERATURE IS LOWER THAN 0°F
- QUALIFY WELDING OPERATORS IN ACCORDANCE WITH A.W.S. D1.5. QUALIFICATION TESTS 2. SHALL BE RUN BY RECOGNIZED TESTING LABORATORY APPROVED BY THE ENGINEER AT THE CONTRACTOR'S EXPENSE
- WELDING OPERATORS SHALL BE SUBJECT TO EXAMINATION FOR REQUALIFICATION USING EQUIPMENT, MATERIALS, AND ELECTRODES EMPLOYED IN EXECUTION OF WORK, SUCH 3. REQUALIFICATION, IF ORDERED BY THE ENGINEER, SHALL BE DONE AT CONTRACTOR'S EXPENSE
- GRIND EXPOSED EDGES OF WELDS TO 1/8" MINIMUM RADIUS. GRIND BURRS, JAGGED EDGES, 4 AND SURFACE DEFECTS SMOOTH.
- PREPARE WELDS AND ADJACENT AREAS SO THERE IS: 5.
 - A. NO UNDERCUTTING OR REVERSE RIDGES ON WELD BEAD.
 - в NO WELD SPATTER ON OR ADJACENT TO WELD OR OTHER AREA TO BE PAINTED OR COATED
 - C. NO SHARP PEAKS OR RIDGES ALONG WELD BEAD.
- 6. GRIND EMBEDDED PIECES OF ELECTRODE OR WIRE FLUSH WITH ADJACENT SURFACE OF WELD BEAD

BOLTS

- SUPERSTRUCTURE CONNECTION BOLTS SHALL BE ASTM F3125 GRADE A325, MINIMUM 7/8" IN 1. DIAMETER, AND THE BOLT TYPE SHALL BE CONSISTENT WITH THE MATERIAL BEING CONNECTED.
- 2. ALL OTHER STEEL-TO-STEEL CONNECTION BOLTS SHALL BE ASTM F3125 GRADE A325 MINIMUM 3/4"
- ANY BOLTS THAT REQUIRE REMOVAL AFTER BEING TIGHTENED TO THEIR PROOF LOAD 3 SHALL BE DISCARDED AND A NEW BOLT SHALL BE INSTALLED.
- ALL BOLT HOLES SHALL BE SUB-DRILLED AND REAMED OR DRILLED FROM THE SOLID. AT NO 4. TIME ARE HOLES TO BE SUB-PUNCHED AND REAMED OR PUNCHED FULL SIZE. ALL HOLES SHALL BE 1/16" LARGER THAN THE SPECIFIED BOLT SIZE UNLESS NOTED OTHERWISE
- DRILL FIELD HOLES FOR BOLTS. DO NOT BURN HOLES. NEW OR ENLARGING HOLES BY USE 5. OF CUTTING TORCH IS CAUSE FOR REJECTION OF ENTIRE MEMBER.
- ALL SHOP BOLTS ARE TO BE INCLUDED IN FABRICATION, BOLTS FOR FIELD ERECTION SHALL 6 BE SUPPLIED BY THE FABRICATOR

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TRANSPORTATION DISTRICT

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Chesterton, Indiana 46304

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STANDARD ABBREVIATIONS

NIC

Abut.

Bldg

Bm

Brg

CB

čĪr

CN

Cs

CSS

CSX

CWR

CY

DC

DI

Dia

DOT

Dwg

Ea

El.

Engr

Exp.

Ftg

Gnd

HDPE

Horiz

HTTO

HMA

Inv.

LC

IН

15

LLT

Max

MH

Min

Misc

Lt

Lin Ft, Lf

Ft

FFBW

Elev., El

Exist., Ex

CMP

Conc

Approx.

Abutment

Abutment	NIC	Not In Contract
Approximate	N.T.S.	Not To Scale
Bottom	No.	Number
Building	OHP	Overhead Power
Bench Mark	PC	Point Of Curve
Bearing	PI	Point Of Intersection
Catch Basin	PITO	Point Of Intersection Of Turnout
Centerline	POB	Point Of Beginning
Clearance	POE	Point Of Ending
Corrugated Metal Pipe	POTO	Power Operated Turnout
Canadian National		Proposed
	Pr, Prop	
Concrete	P.S.	Point Of Switch
Curve To Spiral	PT	Point Of Tangency, Point
Chicago South Shore	PVC	Point Of Vertical Curve
Csx Railroad	PVI	Point Of Vertical Intersection
Continuously Welded Rail	PVT	Point Of Vertical Tangency
Cubic Yard	R	Radius
Degree Of Curve	Rd	Road
Ductile Iron	RE	Railway Engineered, Arema Approved Rail Section
Diameter	Rev.	Revision
Department Of Transportation	RH	Right Hand
Drawing	RR	Railroad
Easting	Rt	Right
Each	R/W, R.O.W.	Right-Of-Way
Elevation	S	South
Engineer	SC	Spiral To Curve
Elevation	SD	Storm Drain
Existing	SE	Super Elevation
Expansion	SF	Square Foot
Front Face of Backwall	Shld.	Shoulder
Footing	SS	Sanitary Sewer
Foot, Feet	Sta.	Station
Gas	ST.	Street, Spiral To Tangent
Ground	Std.	Standard
High Density Polyethylene	Stm, Sd	Storm Sewer, Storm Drain
Horizontal	SW	Switch
Hand Throw Turnout	T	Тор
Hot Mix Asphalt	Temp.	Temporary
Invert	TF	Track Foot, Track Feet
	T.O.	Turnout
Length Of Curve		
Left Hand	TOR	Top Of Rail
Length Of Spiral	TPG	Through Plate Girder
Linear Foot	Trk	Track
Last Long Tie	TS	Tangent To Spiral
Left	Тур.	Typical
Maximum	V	Velocity
Manhole	Var.	Varies
Minimum	VC	Vertical Curve
Miscellaneous	Vert.	Vertical
Mile Post	W	Water, West
Northing	Xover	Crossover
	Xing	Crossing
	YD	Yard

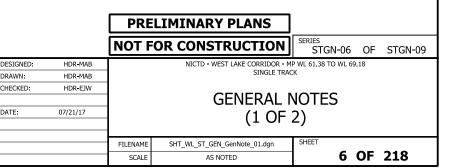
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LEGEND



Planned Boring

Drilled Boring



CAST-IN-PLACE CONCRETE NOTES:

- ALL CAST-IN-PLACE CONCRETE, EXCEPT FOR DRILLED SHAFTS, SHALL BE INDOT CLASS C AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. CAST-IN-PLACE CONCRETE SHALL BE IN ACCORDANCE WITH CHAPTER 8, CONCRETE STRUCTURES AND FOUNDATIONS, OF THE AREMA MANUAL.
- 2. CEMENT SHALL CONFORM TO THE FOLLOWING:
 - A. STANDARD CONCRETE CEMENT SHALL BE PORTLAND CEMENT, TYPE I, IA, CONFORMING TO THE REQUIREMENTS OF ASTM C 150.
 - HIGH-EARLY STRENGTH CEMENT SHALL BE PORTLAND CEMENT, TYPE III OR IIIA, CONFORMING TO THE REQUIREMENTS OF ASTM C 150
 - THE TYPE OF CEMENT SHALL BE AS SHOWN ON THE PLANS OR AS SPECIFIED.
- CONCRETE SEALER SHALL BE APPLIED TO ALL EXPOSED SURACES OF THE PIERS, 3. BENTS, ABUTMENTS, WINGWALLS, AND RETAINING WALLS
- ALL EXPOSED CONCRETE SURFACES SHALL HAVE A SMOOTH TROWELED FINISH 4 AND HAVE NO DEPRESSIONS WHICH HOLD WATER.
- ALL EXPOSED EDGES SHALL HAVE A 3/4" x 3/4" CHAMEER 5
- 6. REINFORCING BARS SHALL MEET THE FOLLOWING REQUIREMENTS:
 - ALL REINFORCING BARS SHALL BE IN ACCORDANCE WITH CHAPTER 8, CONCRETE Α. STRUCTURES AND FOUNDATIONS, OF THE AREMA MANUAL.
 - BARS SHALL BE INTERMEDIATE GRADE, NEW DEFORMED BILLET STEEL, В. CONFORMING TO THE REQUIREMENTS OF ASTM A 615, GRADE 60,
 - C. SIZE, GRADE, SHAPE AND LENGTH SHALL BE AS SHOWN ON THE PLANS.
 - BARS SHALL BE FREE FROM DIRT, PAINT, OIL, GREASE, THICK RUST AND OTHER D. FOREIGN SUBSTANCES.
 - REINFORCING BARS SHALL MEET THE LAP REQUIREMENTS OF AREMA CHAPTER 8 Ε. - CONCRETE STRUCTURES, SECTION 2.14 AND 2.22.3 FOR A CLASS C SPLICE.
 - THE MINIMUM CLEAR DISTANCE FROM THE REINFORCING BARS TO SURFACE OF THE CONCRETE SHALL BE IN ACCORDANCE WITH AREMA CHAPTER 8 - CONCRETE STRUCTURES, SECTION 2.6.1 - MINIMUM CONCRETE COVER
 - REINFORCEMENT BENDING DETAILS SHALL BE IN ACCORDANCE WITH THE G. "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", ACI 315, LATEST EDITION.
 - BARS SHALL BE BENT IN THE PLANE FOR WHICH THEY WERE DESIGNED. MAXIMUM H. ALLOWABLE DEVIATION FOR NO. 7 BARS AND SMALLER SHALL BE 1/2" OUT OF PLANE AND MAXIMUM ALLOWABLE DEVIATION FOR NO. 8 BARS AND LARGER SHALL BE 1 INCH OUT OF PLANE
 - FABRICATION BENDING AND PLACEMENT OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH CURRENT AREMA GUIDELINES FOR CONCRETE STRUCTURES AND FOUNDATIONS.
 - TIE WIRES USED FOR TYING REINFORCING BARS SHALL BE A MINIMUM DIAMETER J. OF NO. 16 GAUGE, BLACK, SOFT IRON WIRE.

WATERPROOFING NOTES:

- 1. STEEL DECKS ARE TO BE WATERPROOFED USING A LIQUID SPRAY APPLIED ELASTOMERIC WATERPROOFING MEMBRANE FROM AN APPROVED SUPPLIER. APPROVED SUPPLIERS INCLUDE RJ WATSON (BRIDGE TECH) AND BRIDGE PRESERVATION, LLC.
- HORIZONTAL SURFACES OF WATERPROOFING MEMBRANE ARE TO BE PROTECTED WITH 2. ASPHALT PANELS OR BALLAST MATS.
- 3. EXPANSION JOINTS ARE TO BE DETAILED FOLLOWING MEMBRANE SUPPLIER'S RECOMMENDATIONS.

RETAINING WALL NOTES:

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- RETAINING WALLS PROPOSED THROUGHOUT THE PROJECT ARE PRECAST MODULAR RETAINING WALLS. PRECAST MODULAR WALL SYSTEMS MUST BE APPROVED BY THE OWNER AND HAVE A PROVEN HISTORY OF USE ON RAILROAD PROJECTS.
- 2. PRECAST MODULAR WALLS MAY BE T-WALL PROVIDED BY THE NEEL COMPANY, EVERGREEN, OR AN APPROVED ALTERNATE.
- BRIDGE SUBSTRUCTURE AND RETAINING WALL UNITS MUST BE DESIGNED AND DETAILED 3. SO THAT THEY ARE COMPATIBLE AND PERFORM AS THEY ARE INTENDED.
- RETAINING WALLS MAY BE PROVIDED WITH A PILE FOUNDATION IF THE ALLOWABLE BEARING CAPACITY OF THE SOIL IS NOT SUFFICIENT FOR THE APPLIED LOAD AND 4. OVEREXCAVATION OR OTHER SOIL REMEDIATION IS NOT FEASIBLE.

FOUNDATION NOTES:

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5.

- SOIL DATA IS AVAILABLE FOR FINAL DESIGN OF BRIDGE FOUNDATIONS. FOUNDATIONS SHOWN IN THE PLANS ARE CONCEPTUAL IN NATURE AND BASED ON VERY LIMITED DATA AND WILL NEED TO BE EVALUATED AND COMPLETED IN FINAL DESIGN.
- MOST BRIDGES ARE EXPECTED TO REQUIRE DEEP FOUNDATIONS. THE PLANS INDICATE PILES 2. AND DRILLED SHAFTS IN SPECIFIC LOCATIONS; HOWEVER THE CONTRACTOR MAY CHOOSE TO REVISE THE FOUNDATION TYPE AT EACH LOCATION TO SUIT THE NEEDS OF THE PROJECT.
- TO ELIMINATE AXIAL REDUCTION FACTORS, A MINIMUM SPACING REQUIREMENT FOR THE 3. PILES OR DRILLED SHAFTS SHOULD BE THREE DIAMETERS (EQUIVALENT) CENTER-TO-CENTER PER AREMA CHAPTER 8, SECTION 4.2.3.3.A (2016). TO ELIMINATE LATERAL REDUCTION FACTORS, A MINIMUM SPACING REQUIREMENT FOR THE PILES OR DRILLED SHAFTS SHOULD BE EIGHT DIAMETERS (EQUIVALENT) CENTER-TO-CENTER.
- ANALYSES THAT MAY NEED TO BE PERFORMED FOR FINAL DESIGN OF FOUNDATIONS INCLUDE 4. LATERAL RESISTANCE, NEGATIVE SKIN FRICTION, SCOUR, AND STATIC PILE ANALYSIS.
 - ASSUMPTIONS USED FOR CONCEPTUAL DESIGN:
 - A. H-PILES
 - THE PILES WHICH ARE ANTICIPATED TO BE DRIVEN TO COMPETENT BEDROCK SHOULD INCLUDE DRIVING TIPS AND/OR SHOES.
 - IF THE PILES ARE DRIVEN TO COMPETENT BEDROCK, IT IS ANTICIPATED THAT 2. THE ULTIMATE GEOTECHNICAL AXIAL RESISTANCE WILL EXCEED THE STRUCTURAL LOAD CAPACITY OF THE PILES. IN THIS CASE, THE STRUCTURAL LOAD CAPACITY WOULD GOVERN THE DESIGN AXIAL RESISTANCE.
 - THE ESTIMATED FACTORED AXIAL RESISTANCE OF A SINGLE HP 14X89 DRIVEN 3 TO BEDROCK IS 165 TONS. THIS CAPACITY IS BASED ON THE REQUIREMENTS OF AREMA CHAPTER 8, SECTION 4.4.2.6 (2016) WITH A REDUCTION FACTOR OF 0.35.
 - THE PILES MAY BE DRIVEN TO REFUSAL ON COMPETENT BEDROCK IF SIDE FRICTION IS NOT ADEQUATE. PILE REFUSAL SHALL BE DEFINED AS 10 HAMMER BLOWS WITH LESS THAN 1-INCH OF PILE PENETRATION.
 - FOR ABUTMENTS AND PIERS NORTH OF DOUGLAS STREET, BEDROCK IS 5. ESTIMATED TO BE BETWEEN ELEVATION 450 AND 500 BASED ON PRELIMINARY INVESTIGATION AND PUBLISHED LITERATURE. PILE LENGTHS ARE ESTIMATED TO BE APPROXIMATELY 100 FEET. FOR ABUTMENTS AND PIERS SOUTH OF DOUGLAS STREET, BEDROCK IS ESTIMATED TO BE BETWEEN ELEVATION 500 AND 525 BASED ON PRELIMINARY INVESTIGATION AND PUBLISHED LITERATURE. PILE LENGTHS ARE ESTIMATED TO BE APPROXIMATELY 80 FEET.
 - B. DRILLED SHAFTS:
 - 1. DESIGN OF DRILLED SHAFTS SHOULD BE IN ACCORDANCE WITH THE CURRENT FHWA DRILLED SHAFT CONSTRUCTION PROCEDURES AND DESIGN METHODS MANUAL.
 - INSTALLATION OF DRILLED SHAFT FOUNDATIONS SHOULD BE MONITORED BY 2 AN EXPERIENCED AND QUALIFIED GEOTECHNICAL ENGINEER FAMILIAR WITH THE SUBSURFACE CONDITIONS AT THE PROJECT SITE. THE MEANS AND METHODS OF CONSTRUCTION ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR'S PROPOSED DRILLED SHAFT INSTALLATION PROCEDURE SHALL BE SUBMITTED FOR APPROVAL BEFORE CONSTRUCTION
 - PERMANENT CASING MAY BE REQUIRED. REQUIREMENTS FOR CASING TO BE 3. DETERMINED IN FINAL DESIGN. IN CASE OF OVERBURDEN SOILS THAT ARE SUBJECT TO SLOUGHING OR CAVING. TEMPORARY CASING MAY BE REQUIRED IN ORDER TO EXCAVATE AND DRILL OR CORE THE DRILLED SHAFT. TEMPORARY CASING, IF USED, SHOULD BE EXTRACTED IN STAGES ENSURING THAT AFTER EACH LENGTH OF CASING IS REMOVED, THE CONCRETE LEVEL IS BROUGHT BACK UP TO THE GROUND LEVEL BEFORE THE NEXT LENGTH IS REMOVED
 - DRILLED HOLES MUST NOT BE LEFT OPEN OVERNIGHT. 4
 - FINAL CLEANOUT OF THE DRILLED SHAFT SHOULD BE PERFORMED USING AN 5. AIR-LIET OR CLEANOUT BUCKET AFTER DRILLING IS COMPLETE AND PRIOR TO INSTALLING THE REINFORCEMENT. THE HOLE SHOULD BE DEWATERED PRIOR TO CONCRETE PLACEMENT.
 - 6. STEEL REINFORCEMENT SHOULD BE SET AND SECURED IN PREDRILLED HOLES AND ENCASED IN CONCRETE IMMEDIATELY FOLLOWING THE COMPLETION OF DRILLING.
 - CARE SHOULD BE TAKEN TO PROTECT BARE STEEL FROM BEING PLACED IN 7. DIRECT CONTACT WITH THE POTENTIALLY CORROSIVE NATIVE ON-SITE SOIL WITHOUT PROVIDING ADDITIONAL PROTECTIVE MEASURES.
 - CONCRETE SHOULD HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS. IT IS RECOMMENDED THAT CLASS A MODIFIED CONCRETE BE USED IN THE CONSTRUCTION OF THE DRILLED SHAFTS. THE CONCRETE SHOULD EXHIBIT GOOD WORKABILITY AND SHOULD BE PLACED WITHIN ONE HOUR OF MIXING.





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ISSUE	DATE	DESCRIPTION	
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DYER TO HAMMOND, INDIANA

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ESIGNED: RAWN:

	PRE	LIMINARY PLANS	
	NOT F	OR CONSTRUCTION	SERIES STGN-07 OF STGN-09
HDR-MAB		NICTD - WEST LAKE CORRIDOR - M	
HDR-MAB		SINGLE TRAC	CK
HDR-EJW			
		GENERAL N	NOTES
07/21/17		(2 OF	2)
			<u> </u>
			SHEET
	FILENAME	SHT_WL_ST_GEN_GenNote_02.dgn	
	SCALE	AS NOTED	7 OF 218

SUMMARY OF QUANTITIES

	202-51330	206-51225	206-XXXXX	211-09265	604-92237	616-02320	701-02936	701-06011	701-09557	701-09559	701-09684	701-09739	701-09891	701-11898	701-11899	701-51195	701-95782	702-02925	702-51001	702-51005	702-51015	703-06029	707-XXXX
LOCATION	PRESENT STRUCTURE, REMOVE	EXCAVATION, DRY	SHEET PILING, PZC 39	STRUCTURE BACKFILL, TYPE 2	HAND RAIL, STEEL	GEOTEXTILES	PILE, STEEL H, HP 14 X 89	DYNAMIC PILE LOAD TEST	TEST PILE, DYNAMIC, PRODUCTION	TEST PILE, DYNAMIC, RESTRIKE	PILE SHOE, HP 14 X 89	PILE SHOE, HP 12 X 53	PILE SHOE, HP 12 X 84	PILE, STEEL H, HP 14 X 117		PILE, STEEL H, HP 12 X 53		WATER- PROOFING MEMBRANE SYSTEM	CONCRETE, A, SUPER- STRUCTURE	CONCRETE, A, SUB- STRUCTURE	CONCRETE, B, FOOTINGS	REINFORCING BARS, EPOXY COATED	G PPC SINGL CELL BOX GIRDER, 3 IN.
Bridge 101 - CSX RR over Main Street	0	78) 36	154	0	0) 0	0	0	0	0	0	0	0	0	1,746	0	97	0	15,714	
Bridge 101 - Pedestrian Bridge over Main Street	0) 27		0	0		2 180	0	0	Ŭ	0	0	0	480	0	· · · ·	0	53		8,586	
Retaining Walls 301S & 301N	0		72,463		1,398	0	0		100		0	v	0	0	0		0	, v	0	0	0	0,500	<u> </u>
Bridge 102 - CSX RR/NICTD over Pedestrian Underpass	0	-	/2,403	35 0		0	2,754	-	2 180	Ů	34		0	2,754	34		0		0	240	0	20,000	<u> </u>
					33	0	2,754	2	2 180	2			0	2,/54			0	0	0		0	38,880	4
Retaining Wall 301A	0	-	(10,359	1,250	0	0		0	0	0	51	0	0	0	4,590	0		0	0	0	0	<u> </u>
Retaining Walls 302W & 302E	0	-	(22,438	2,595	0	0	-	0 0	, v	0	100		0	0	5,150	0	- · · ·	0	0	0	0	
Elevated Structure 201	0			99	1,165	0	480		3 720		6	Ŭ	0	3,192	42		0	12,033	0	322			
Bridge 103 - NICTD over Industrial Spur	0) 56	222	0	720	2	2 180		9	v	26		0	-	2,145		0	85			
Bridge 104 - NICTD over 45th Street	0	48	-) 12	151	0	0	1	L 90		0	v	13	0	0	0	1,053		0	32			
Elevated Structure 202	0	280		94	1,603	0	4,740	10	900	10	60	0	0	0	0	0	0	17,941	0	362	187		
Bridge 105 - NICTD over CSX RR	0			0 0	0	0	0	C	0 0	0	0	0	0	0	0	0	0	2,799	0	295	0	47,790	
Elevated Structure 203	0	252	() 84	1,456	0	5,266	10	900	10	54		0	0	0	0	0	16,249	0	366	168	86,508	1
Retaining Walls 303W & 303E	0	0	0	9,264	1,218	0	0	(C	0 0	0	0	51	0	0	0	4,590	0	0	0	0	0	0	<u> </u>
Pedestrian Tunnel 401 at Pennsy Greenway	0	0	(0 0	32	0	0	C	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Retaining Walls 304W & 304E	0	0	0	0 0	921	0	0	C	0 0	0	0	39	0	0	0	3,510	0	0	0	0	0	0	<u> </u>
Retaining Walls 305W & 305E	0	0	(2,143	1,003	0	0	C	0 0	0	0	42	0	0	0	3,780	0	0	0	0	0	0	
Bridge 106 - NICTD over Broadmoor Avenue	0	0	0	0 0	123	0	1,080	2	2 180	2	12	0	0	0	0	0	0	1,371	0	48	0	7,776	L
Retaining Walls 306W & 306E	0	0	0	9,135	3,043	0	0	C	0 0	0	0	123	0	0	0	11,070	0	0	0	0	0	0	
Pedestrian Tunnel 402 at Beldon Place	0	0	0	0 0	24	0	0	C	0 0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bridge 107 - NICTD over Little Calumet River	1	569	0	285	0	0	8,640	4	i 360	4	96	0	0	0	0	0	0	1,161	0	265	143	66,096	1
Bridge 107A - Pedestrian Bridge over Little Calumet River	0	161	0	81	0	0	0	e	540	6	0	0	0	0	0	0	0	0	0	33	63	15,552	
Retaining Walls 308W & 308E	0	0	(4,882	1,795	0	0	C	0 0	0	0	72	0	0	0	6,480	0	0	0	0	0	0	
Bridge 108 - NICTD over Fayette Street	0	0	0	0 0	0	0	1,680	C	0 0	0	16	0	0	0	0	0	0	570	0	40	0	6,480	
Retaining Walls 309W & 309E	0	0	0	3,993	583	0	0	c	0 0	0	0	24	0	0	0	2,160	0	0	0	0	0	0	
Bridge 109 - NICTD over Sibley Street	0	0	0) 40	214	0	3,600	C	0 0	0	40	0	0	0	0	0	0	2,778	0	93	0	15,066	
Elevated Structure 204	0	181	0) 61	501	0	1,536	4	ł 360	4	16	0	0	0	0	0	0	5,615	0	160	117	44,874	
Bridge 109A - NICTD over State Street	0	0	0	0 0	201	0	0	0) 0	0	0	0	0	0	0	0	0	2,136	0	56	0	9,072	
Bridge 110 - NICTD over NS RR	0	22	0	0 0	0	0	0	0) 0	0	0	0	0	0	0	0	0	,	0	576		93,312	
Bridge 111 - NICTD over Willow Court	0		(0 0	0	0	0	0	0 0	0	0	0	0	0	0	0	0	1,670	0	45		7,290	
Bridge 112 - NICTD over IHB RR	0	8	(0	0	0	0	, r) 0	0	0	0	0	0	0	0	0	413	0	43	-	6,966	
Elevated Structure 207	0	480	(0 160	1,253	0	900	12	2 1,080	12	10	0	0	0	0	0	0	13,450	0	528		85,536	
Bridge 113 - NICTD over Hohman Avenue	0			5 58	0	0	0) _,000	0	0		0	0	0	0	0	4,571	0	380	0	61,560	
Elevated Structure 207A	0) 0	1,111	0	4,950		3 720	- · · ·	55	v	0	0	0		0		0	77	0	12,474	
Bridge 115 - NICTD over Grand Calumet River	0				1,111	0	3,600		2 180		40		0	0	0	0	0	3,723	0	85		12,474	
Retaining Walls 312W & 312E	0) 0 7,511	2,198	0	 		- 100	2	40	90	0	0	0	8,100	0	3,723	0	0	0	13,770	1
Elevated Structure 208	0	-) 7,511	2,198	0	8,254		5 540	0	86	90	0	0	0	0,100	0	8,772	763	, v	371	117.612	1
Elevated Structure 208 Bridge 116 - NICTD over CSX RR	0) 75		0	8,254		540 0		86	0	0	0	0	0	0	· · · · ·		130		· · · ·	
Elevated Structure 209	0) 21			12,976	-	810	, v	136	-	0	•	0		0					,	
	0						12,9/6		018 0		136		•	0	0	-	0	14,394	1,004	353	593	153,252	
Retaining Walls 313W & 313E	-			7,264			0			0	0	63	0	0	0	5,670	0	0	0		0	0	.+
Pedestrian Tunnel 403 at Gateway Station	0	0		0 0	34	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	
TOTAL	1 LS	4,322 CYS	72,463 SFT	78,332 CYS	28,380 LFT	345,721 SYS	61,176 LFT	88 EACH	7,920 LFT	88 EACH	670 EACH	723 EACH	39 EACH	5,946 EACH	76 EACH	65,010 EACH	3,198 EACH	132,306 SFT	1,767 CYS	5,119 CYS	1,869 CYS	1,132,056 LBS	461 LFT

F R	
HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chlcago, IL 60631	
www.hdrinc.com	

ISSUE	DATE	DESCRIPTION





DESIGNED: DRAWN: CHECKED: DATE:

DYER TO HAMMOND, INDIANA

	PRE	LIMINARY PLANS			
	NOT F	OR CONSTRUCTION	SERIES STGN-08	OF	STGN-09
HDR-MAB		NICTD - WEST LAKE CORRIDOR - M		9.18	
HDR-MAB		SINGLE TRAC	CK		
HDR-EJW					
		SUMMARY OF Q	UANTITI	ES.	
07/21/17		(1 OF			
			Z)		
			SHEET		
	FILENAME	SHT_WL_ST_GEN_QuantSum_01.dgn		~-	210
	SCALE	AS NOTED	8	OF	218

SUMMARY OF QUANTITIES

	1 700 E1001	711 51025	714 2222	714 2222	714 2222	715 05052	720 11674	720 11677	720 11722	770 2222	720 11056	721 02047	VVV VVVVV	VVV VVVVV		VVV VVVVV		VVV VVVV	v
	/09-51821	711-51035			714-XXXXX		/28-116/4	/28-116/7	/28-11/33	/28-XXXXX	/28-11856	/31-9394/	7XX-XXXXX	xxx-xxxxX	8 IN. DIA	XXX-XXXXX	6 IN.	XXX-XXXX	<u> </u>
		l.			, STRUCTURE,										HALF-ROUND		NON-		
LOCATION	SURFACE	CTDUCTUDAL			CONCRETE,	PIPE,	DRILLED	DRILLED	DRILLED	DRILLED	DRILLED	LEVELING	PRECAST MODULAR	CONCRETE	BITUMINOUS	STEEL	PERFORATED	ROPE	
LOCATION	SEAL	STRUCTURAL STEEL	BOX	BOX	BOX			SHAFT 84 IN.				PAD,	RETAINING	STAIN	COATED	GRATE	CORRUGATED		
	JLAL	JILL			SECTION, 15	OUTLET 6 IN.	DIAMETER	DIAMETER	DIAMETER	DIAMETER	DIAMETER	CONCRETE	WALL	STAIN	GALVANIZED	WALKWAY		IN. DIA.	
		l.			FT. X 10 FT								VV/ LL		PERFORATED		PIPE		
		I													DRAIN PIPE		DOWNSPOUT		
		I																	_
Bridge 101 - CSX RR over Main Street	1437		5 (0	0 0	0 0	0	0 0	320	180	0	0	0	0	0	0	0 0		0
Bridge 101A - Pedestrian Bridge over Main Street	1128	0	0 0	0	0 0	0 0	0	0 0	0	0	0	0	0	0	0	0	0 0		0
Retaining Walls 301S & 301N	0	0	0 (0	0 0	2796	0	0 0	0	0	0	33	0	17558	0	0	0 0		0
Bridge 102 - CSX RR/NICTD over Pedestrian Underpass	2613	86529	9 (0	0 0	0 0	0	0 0	0	0	0	0	0	0	0	0	0 0		0
Retaining Wall 301A	0	0	0 0	0	0 0	1250	0	0 0	0	0	0	29	24713	0	10	0	10		0
Retaining Walls 302W & 302E	0	0	0 0	0	0 0	2636	0	0 0	0	0	0	62	75195	57682	0	0	0 0		0
levated Structure 201	11118	1521658.6	5 (0	0 0	0 0	0	0 0	480	0	0	0	0	0	0	0	0 0		0
ridge 103 - NICTD over Industrial Spur	2063	415729.6	5 (0	0 0	0	0	0 0	0	0	0	0	0	0	0	0	0		0
Bridge 104 - NICTD over 45th Street	1403	213174.5	5 (0	0 0	0 0	0	0 0	80	0	0	0	0	0	0	0	0 0		0
levated Structure 202	14888	2095291	1 (0	0 0	0 0	0	0	800	0	0	0	0	0	0	0	0 0		0
ridge 105 - NICTD over CSX RR	2940			0	0 0	0 0	0	316	0	0	320	0	0	0	0	636	0		0
evated Structure 203	13525			0	0 0	0 0	0	0 0	0	0	0	0	0	0	0	C	0 0		0
etaining Walls 303W & 303E	0	(0	0 0	1655	0	0 0	0	0	0	30	32211	23107	0	0	0 0		0
destrian Tunnel 401 at Pennsy Greenway	0	, r	0 42	2	0 0	32) 0	0	0	0	0	0	0	0) 0		0
etaining Walls 304W & 304E		r		0) 921			n 0	0	0	21	12584	7126	0				
	0			0) 521			0	0	0	0	12304	, 120		0			
etaining Walls 305W & 305E	0	0				868			0	0	0	24	15524	9366	0	0			
	1143	140552.5		0					0	0	0	24	15524	0066	0				
ridge 106 - NICTD over Broadmoor Avenue	1143	140552.5		0					0	0	0	73	0	20754	0				
etaining Walls 306W & 306E	0	0				3043			0	0	0	/3	55403	28754	0	0			
edestrian Tunnel 402 at Beldon Place	0	0) (0 2		24			0	0	0	0	0	0	0	0			
idge 107 - NICTD over Little Calumet River	3031) اد ا			0	0	0	0	0	0	0	0	0	0	851	0		
ridge 107A - Pedestrian Bridge over Little Calumet River	2415	0	U] (U	U] (0	0	0	0	0	0	0	0	0	0	0	0		0
	0	0		0	0 0	0	0	0	0	0	0	0	0	0	0	0	0		0
etaining Walls 308W & 308E	0	0	0 (0	0 0) 1795	0	0	0	0	0	34	21241	14543	0	0	0		0
ridge 108 - NICTD over Fayette Street	2204	295696.5	5 (0	0 0	0 0	0	0	0	0	0	0	0	0	0	448	0	22	21
etaining Walls 309W & 309E	0	0	0 (0	0 0	558	0	0 0	0	0	0	15	18465	15469	0	0	0 0		0
ridge 109 - NICTD over Sibley Street	2372	390740.9	9 (0	0 0	0 0	0	0 0	0	0	0	0	0	0	0	C	0 0		0
levated Structure 204	4896	451289.3	3 (0	0 0	0 0	0	0 0	0	0	360	0	0	0	0	0	0 0		0
ridge 109A - NICTD over State Street	1958	324117.2	2 (0	0 0	0 0	0	0	80	0	0	0	0	0	0	0	0 0		0
ridge 110 - NICTD over NS RR	5427	963359.1	1 (0	0 0	0	360	396	0	0	0	0	0	0	0	1202	2 0		0
Bridge 111 - NICTD over Willow Court	1542	278070.1	1 (0	0 0	0 0	0	0 0	90	0	160	0	0	0	0	0	0		0
Bridge 112 - NICTD over IHB RR	2200	244795.1	1 (0	0 0	0	0	0 0	108	0	0	0	0	0	0	245	5 O		0
levated Structure 207	10333	1140679.1	1 (0	0 0	0 0	0	0 0	1080	0	0	0	0	0	0	0	0 0		0
Bridge 113 - NICTD over Hohman Avenue	4752	942245.7	7 (0	0 0	0 0	0	0 0	340	0	0	0	0	0	0	1282	2 0		0
levated Structure 207A	11451	1445467.1	1 (0	0 0	0 0	0	0 0	0	0	0	0	0	0	0	0	0 0		0
ridge 115 - NICTD over Grand Calumet River	4189	706905.1	1 (0	0 0	0 0	0	0 0	0	0	0	0	0	0	0	1055	5 0		0
etaining Walls 312W & 312E	0	(o i	0	0 0	2198	0	0 0	0	0	0	55	47878	33752	0	0	0 0		0
levated Structure 208	9855	1174660.3	3 (0	0 0) 0	0	0 0	0	0	0	0	0	0	0	0	0 0		0
ridge 116 - NICTD over CSX RR	4163		-	0	0 0) 0	0) 0	360	0	0	n	0	0	0	1191			0
levated Structure 209	18008		-	0) 0	n 1) 0	0	0	n	n	0	0	0) 0		0
etaining Walls 313W & 313E	0			0	0 0) 1521	1) n	n	0 0	n 0	38	26858	17494	0	0 0) n		0
edestrian Tunnel 403 at Gateway Station	0	r		0	0 34) n	n	0	0	0	0.000	0	0				
sassanan ranner ros ac saceway stadon					<u> </u>	. 54	1		0	0	0	0	0	0			0		
		18856101.8	42	26	34	19331	360	712	3738	180	840	414	330072	224851	10	6910	10	221	
TOTAL	141054						1 500	1 / 12	0,00	100	1 010	TIT	550072	227031	10	0910	10	1 441	

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ISSUE	DATE	DESCRIPTION



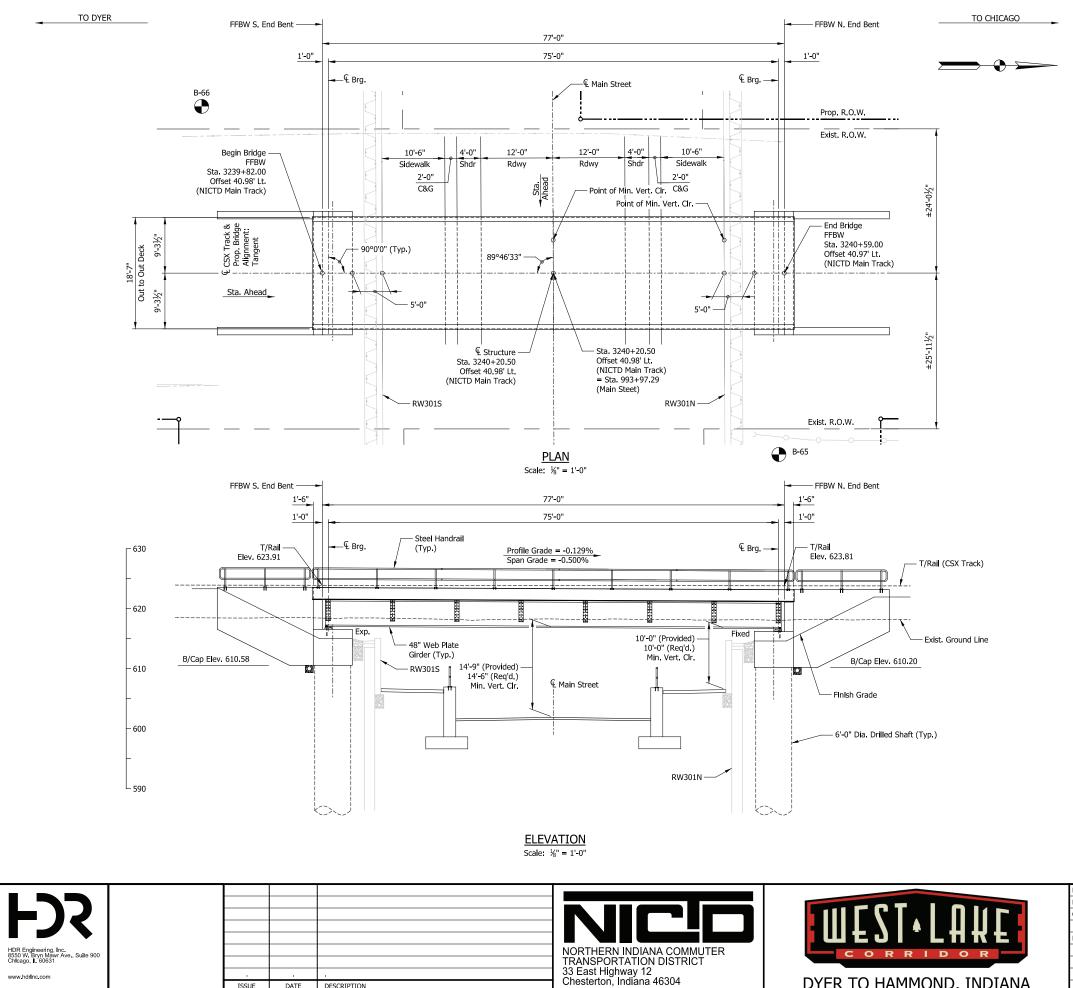


SCALE

AS NOTED

9 OF 218

DYER TO HAMMOND, INDIANA



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ISSUE

DATE DESCRIPTION

DESIGN DATA

SPECIFICATIONS: American Railway Engineering and Maintenance-of-Way Association (A.R.E.M.A.), Manual for Railway Engineering, 2016, Chapter 8 and 15.

LIVE LOADS: Cooper E-80 or AREMA Alternate Live Load

DEAD LOADS: Maximum Ballast Depth = 21 inches (at High End of Span)

DESIGN STRESSES: Steel: fy = 50,000 psi (ASTM A709 Gr 50) Reinforcing: fy = 60,000 psi (ASTM A615) Concrete: f'c = 4,000 psi (Min.)

SHEET INDEX

BR101-01	General Plan & Elevation
BR101-02	Typical Details
BR101-03	Substructure Details
BR101-04	Framing Plan

CONSTRUCTION DEPTH

Rail (115 Lbs)	6%"
Tie Plate	34"
Tie	7"
Ballast (min.)	8"
Grout	4½" (at Center), 3½" (Min.)
Waterproofing	1"
Deck Plate	58"
Top Flange	3"
Web	48"
Bottom Flange	3"

82.5"

NOTES

DRAWN:

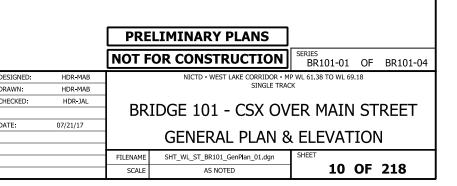
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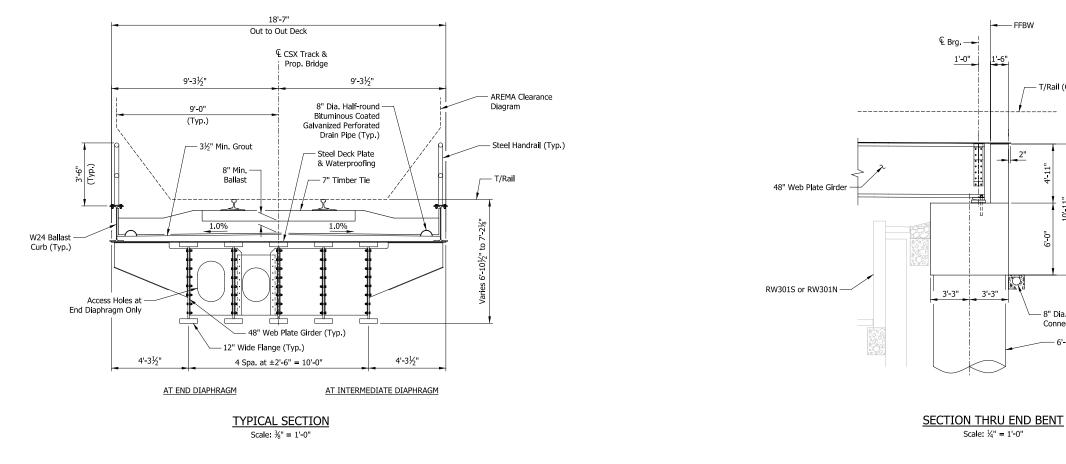
DYER TO HAMMOND, INDIANA

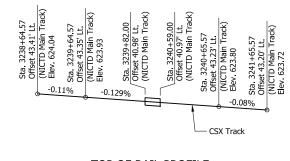
1. For track geometry, see Track Plans.

CSX BRIDGE NOTES

- 1. This structure is designed to carry CSX design criteria Cooper E-80 live load.
- 2. The deck is crowned with a transverse slope of 1" and an underlayment of portland cement concrete with a minimum thickness of $3\frac{1}{2}$ " per CSX design standard.







TOP OF RAIL PROFILE Not to Scale

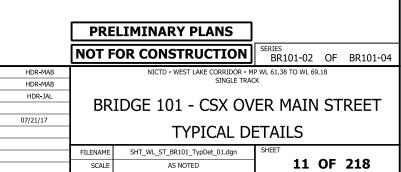
DESIGNED: DRAWN: HECKED: DATE: NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT 33 East Highway 12 Chesterton, Indiana 46304 HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631 ORRIDOR www.hdrinc.com DYER TO HAMMOND, INDIANA ISSUE DATE DESCRIPTION

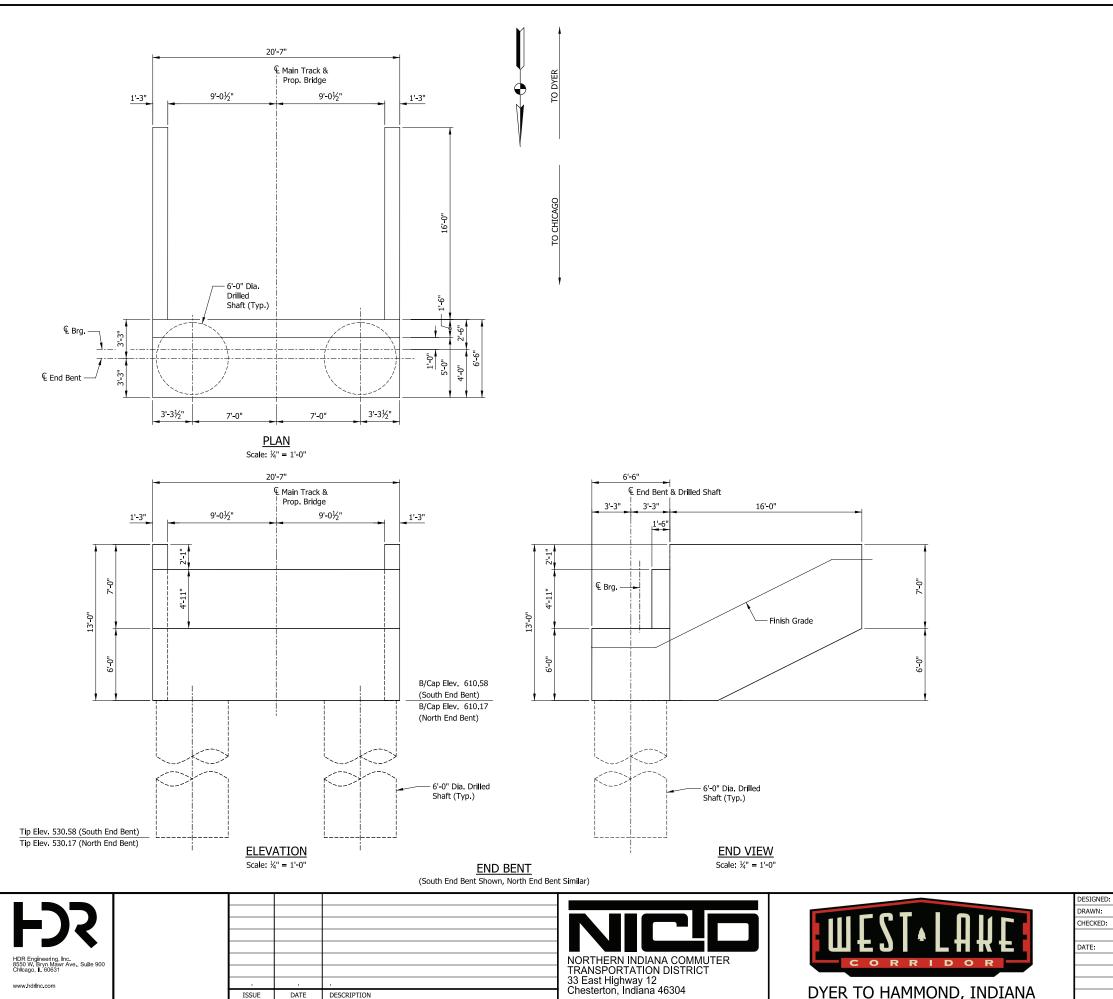
— T/Rail (CSX Track)

10'-1

- 8" Dia. Perforated Pipe Connected to Proposed Storm Sewer

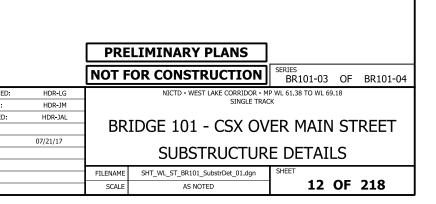
- 6'-0" Dia, Drilled Shaft

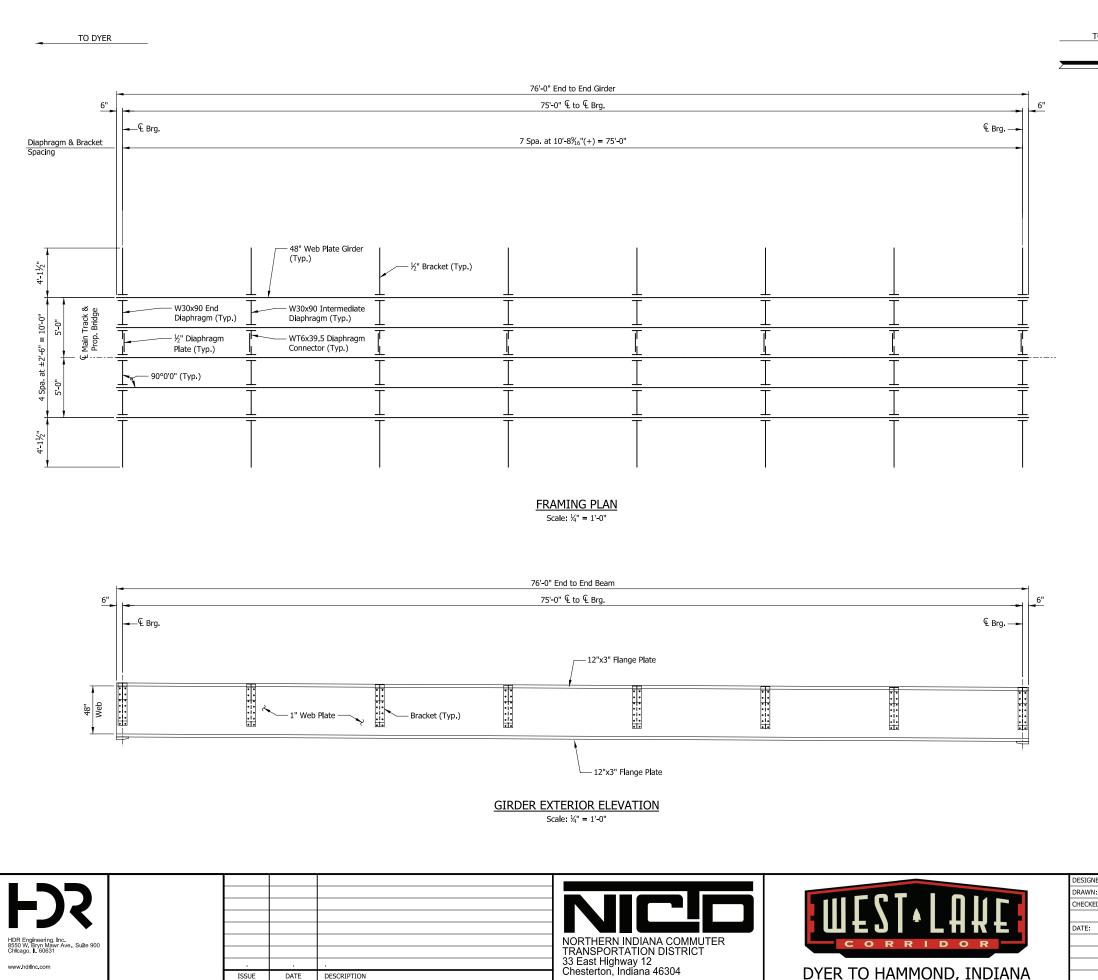




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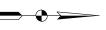


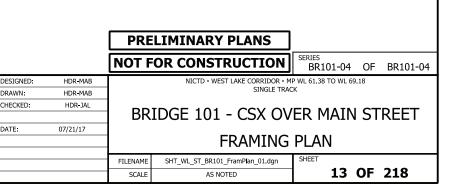


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DATE DESCRIPTION

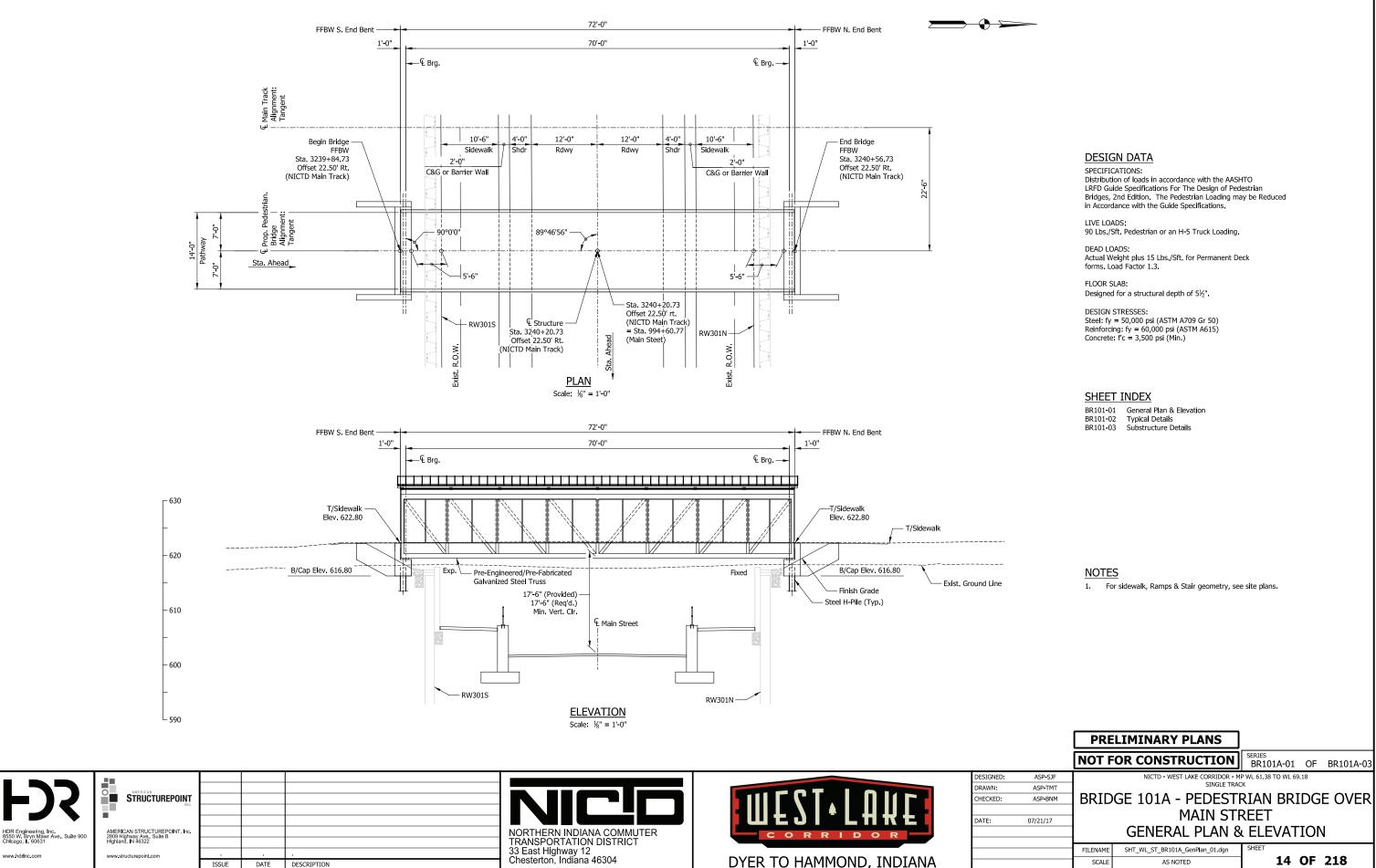
TO CHICAGO





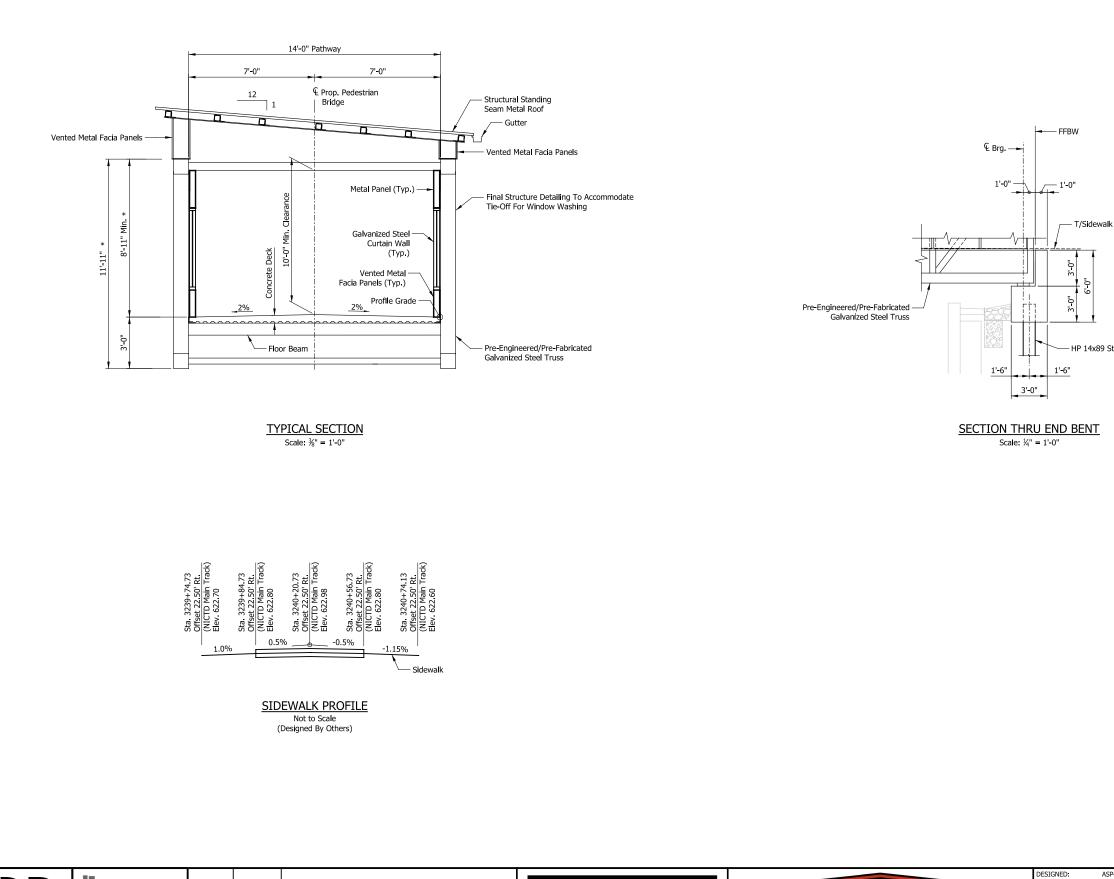


TO CHICAGO



DATE PLOT

BR101-01	General Plan & Elevation
BR101-02	Typical Details
BR101-03	Substructure Details



November 08, 2017 6:03:17 PM JMIGUS DATE РГОТ

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	Chicago, IL 60631	Highland, IN 46322				TRANSPORTATION DISTRICT	
	www.hdrinc.com	www.structurepoint.com				33 East Highway 12 Chesterton, Indiana 46304	
			ISSUE	DATE	DESCRIPTION	Chesterton, Indiana 46304	DYER TO HAMMOND, INDIANA

- HP 14x89 Steel Pile

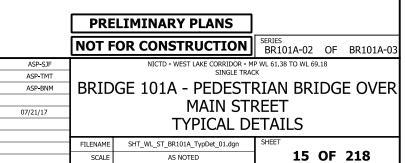
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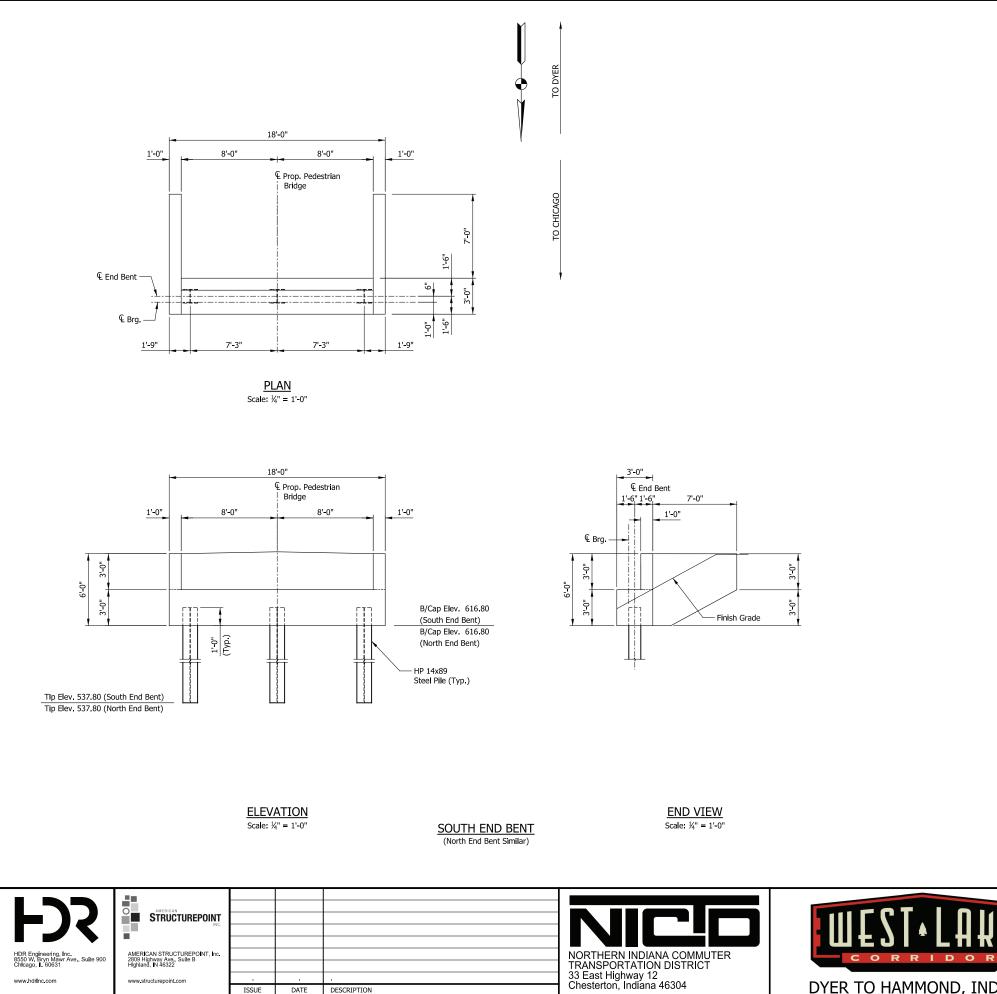
DATE:

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NOTES

1. * Estimated. Actual Dimensions To Be Determined By Fabricator's Design.





ISSUE

DATE DESCRIPTION

	PRE	LIMINARY PLANS				
	NOT F	OR CONSTRUCTION	SERIES BR101A-03	OF	BR101A-03	
ASP-SJF		NICTD - WEST LAKE CORRIDOR - M		9.18		
ASP-TMT		SINGLE TRAC				
ASP-BNM	l BRID	GE 101A - PEDESTI	rian Bri	[DG	e overi	
		MAIN ST			-	
07/21/17						
	SUBSTRUCTURE DETAILS					
	FILENAME	SHT_WL_ST_BR101A_SubstrDet_01.dgn	SHEET			
	SCALE	AS NOTED	16	OF	218	

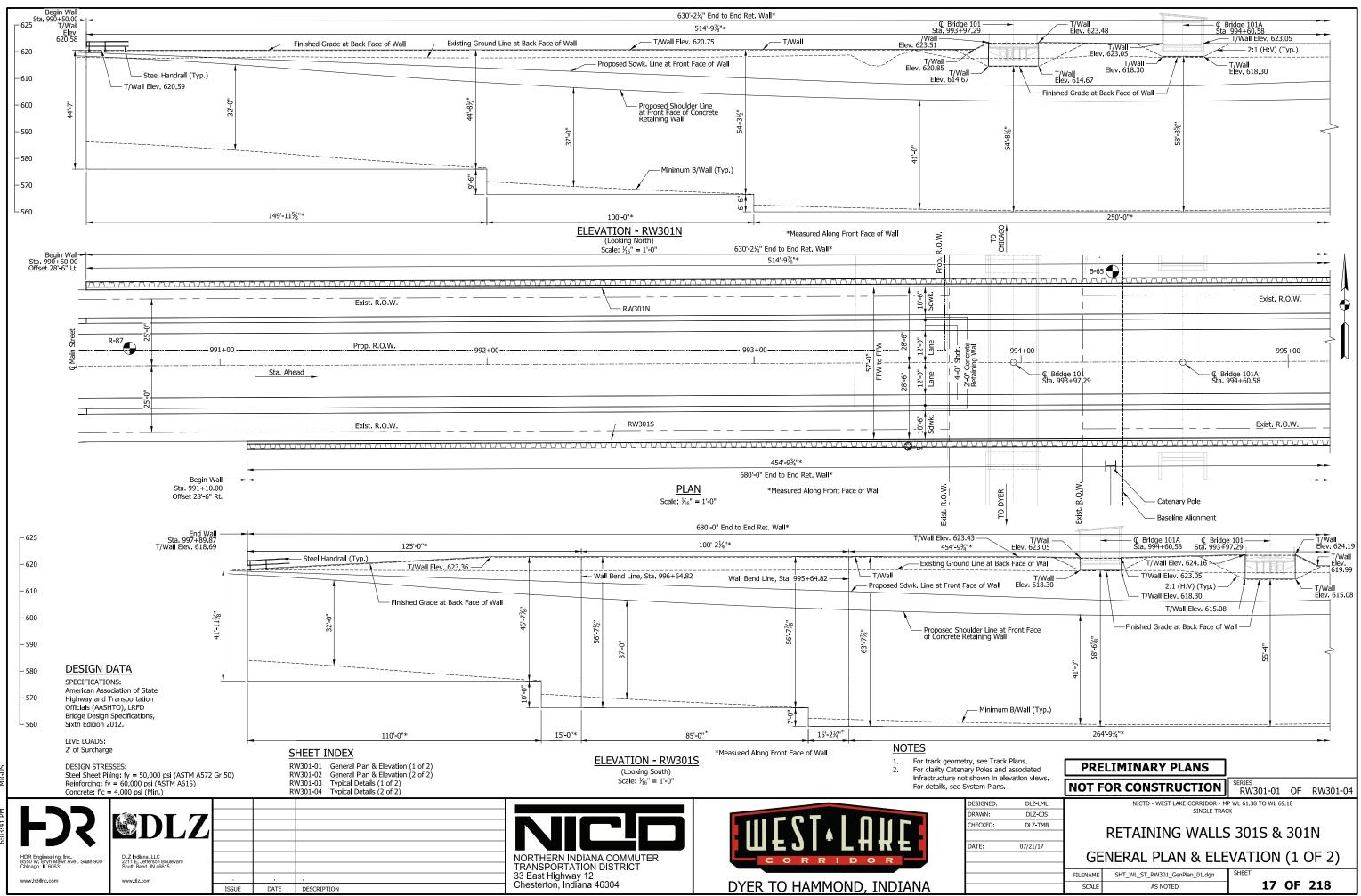
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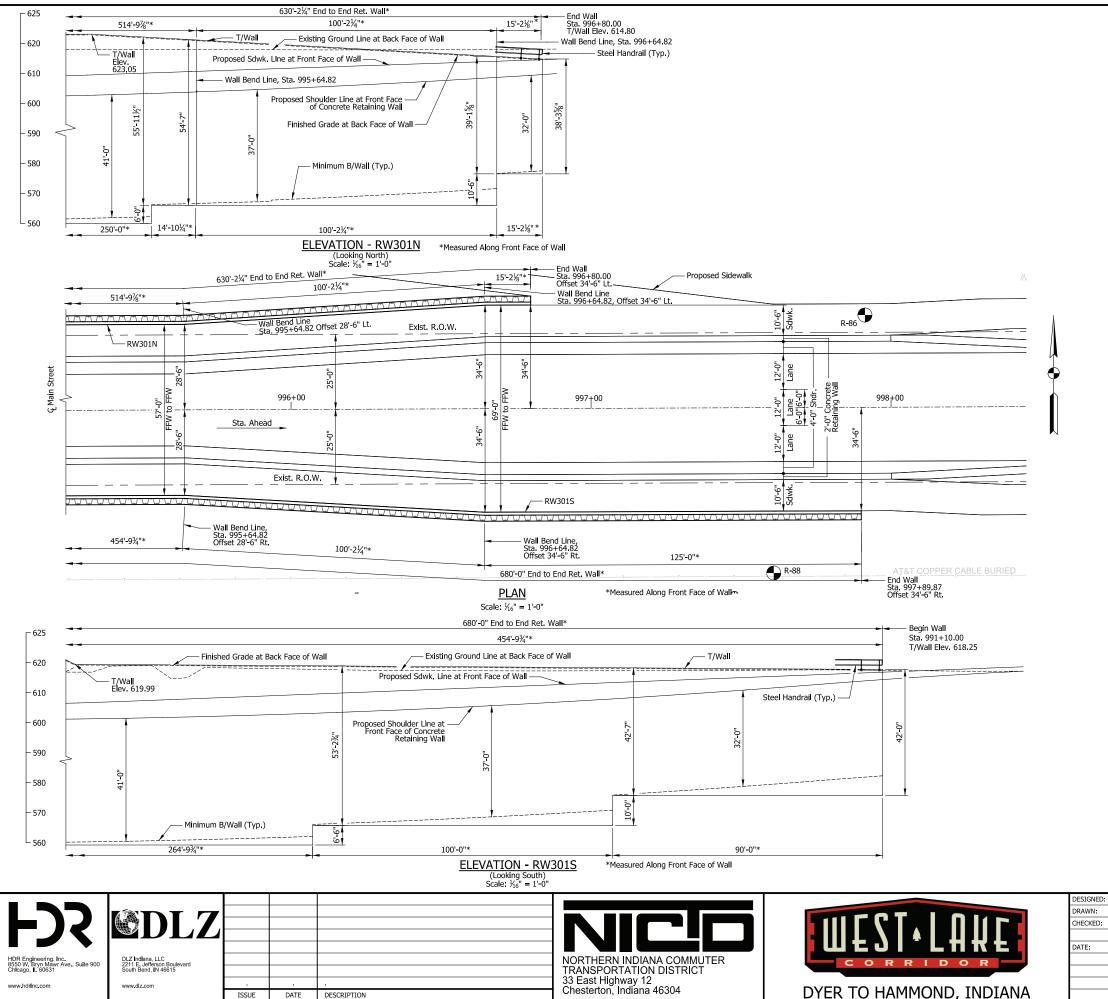
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DATE:

DYER TO HAMMOND, INDIANA



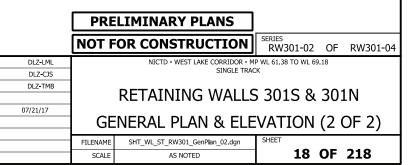


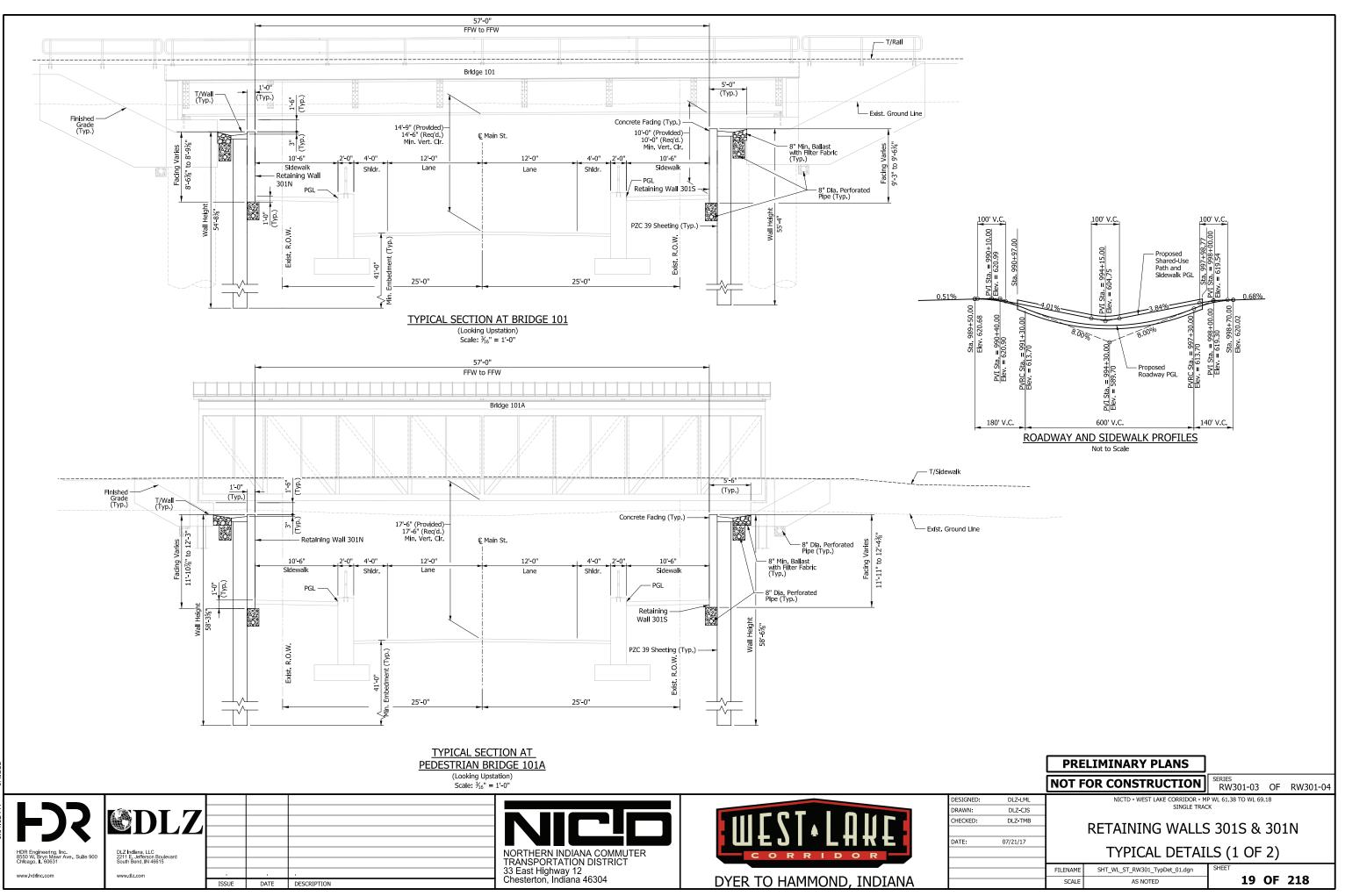
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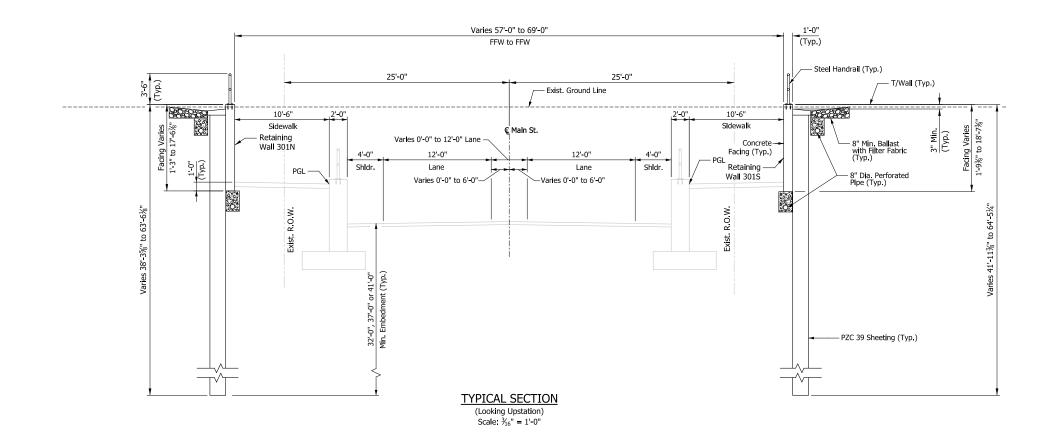
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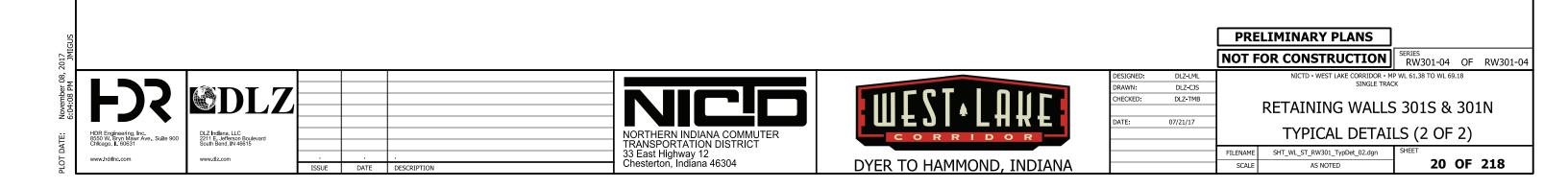
- 1. For track geometry, see Track Plans.
- 2. For clarity Catenary Poles and associated infrastructure not shown in elevation views. For details, see System Plans.

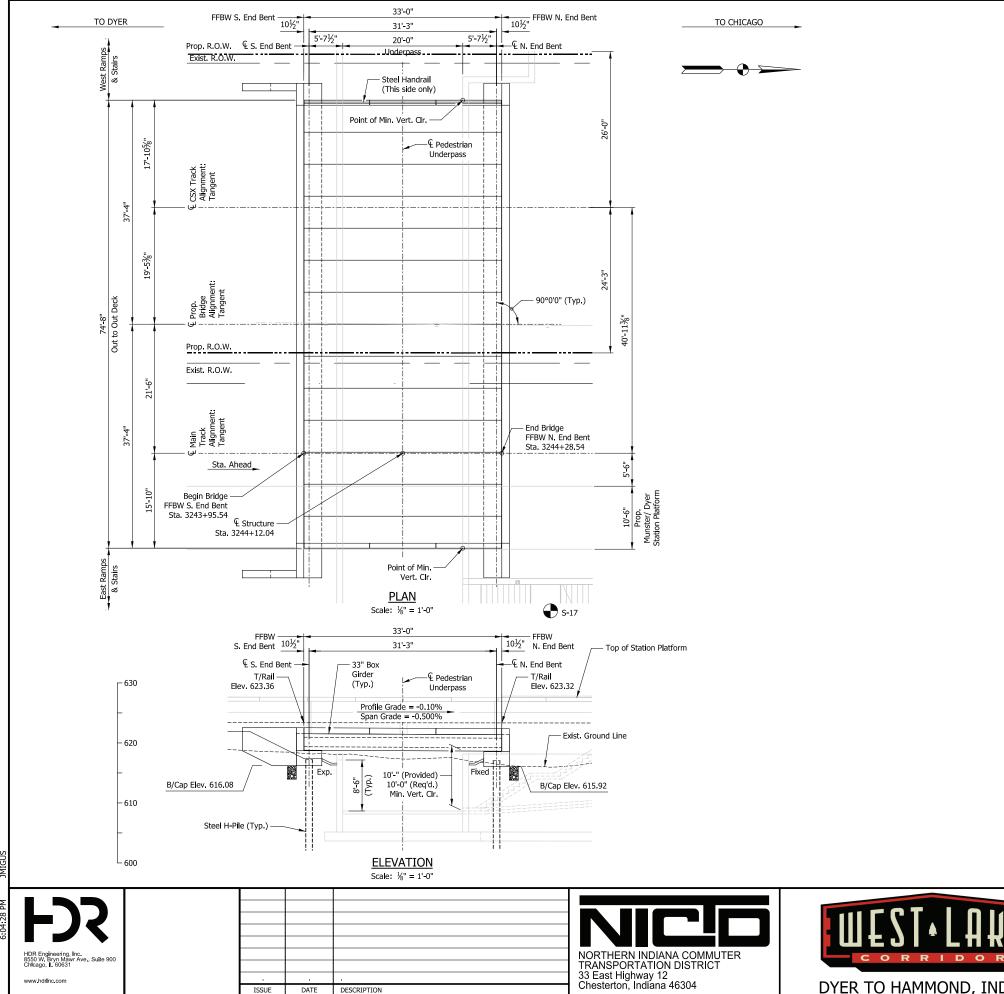




PLOT DATE: November 08, 201 6:04:02 PM 7







DATE PLOT

DYER TO HAMMOND, INDIANA

DESIGN DATA

SPECIFICATIONS: American Railway Engineering and Maintenance-of-Way Association (A.R.E.M.A.), Manual for Railway Engineering, 2016, Chapter 8 and 15.

LIVE LOADS: Cooper E-80 or ARMEA Alternate Live Load

DEAD LOADS: Maximum Ballast Depth = 21 inches (at High End of Span)

DESIGN STRESSES: Steel: fy = 50,000 psi (ASTM A709 Gr 50) Reinforcing: fy = 60,000 psi (ASTM A615) Concrete: f'c = 4,000 psi (Min.)

SHEET INDEX

cal Details
structure Details (1 of 2)
structure Details (2 of 2) ning Plan

CONSTRUCTION DEPTH

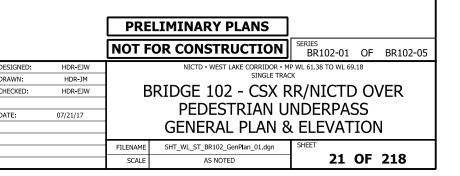
Rail (115 RE)	6%"
Tie Plate	34"
Tie	7"
Ballast (min.)	8"
Waterproofing	1"
Box Girder	33"

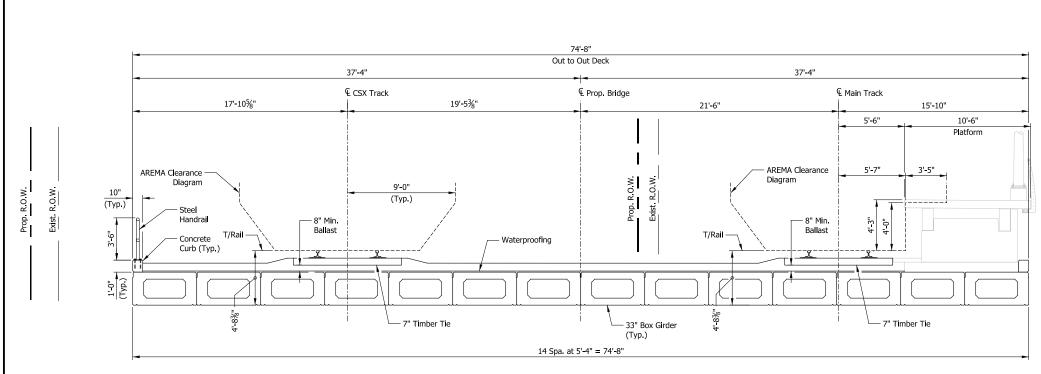
56%"

NOTES

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- For track geometry, see Track Plans.
 For details of Pedestrian Underpass, Ramps & Stairs, see PT400 sheets.

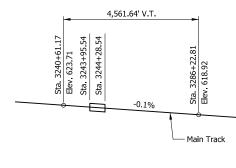




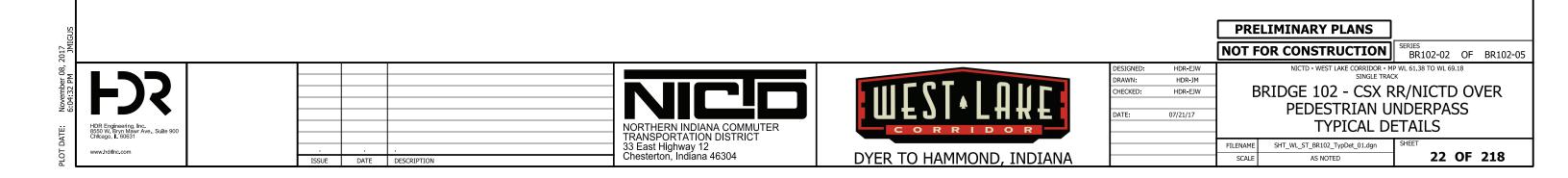
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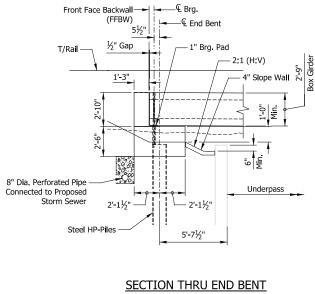
 (Looking North)

 Scale: ¼" = 1'-0"

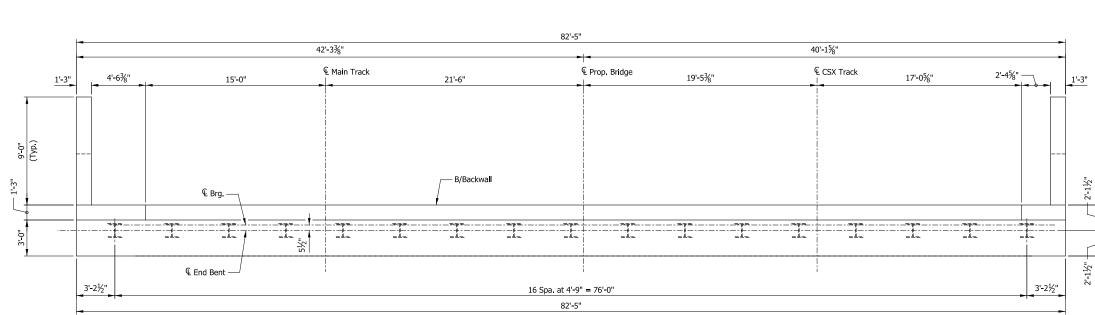


TOP OF RAIL PROFILE Not to Scale

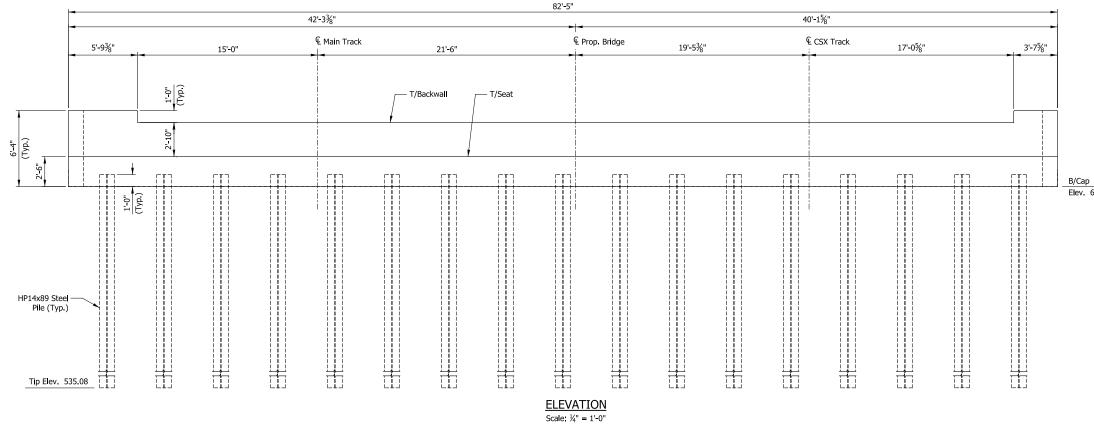




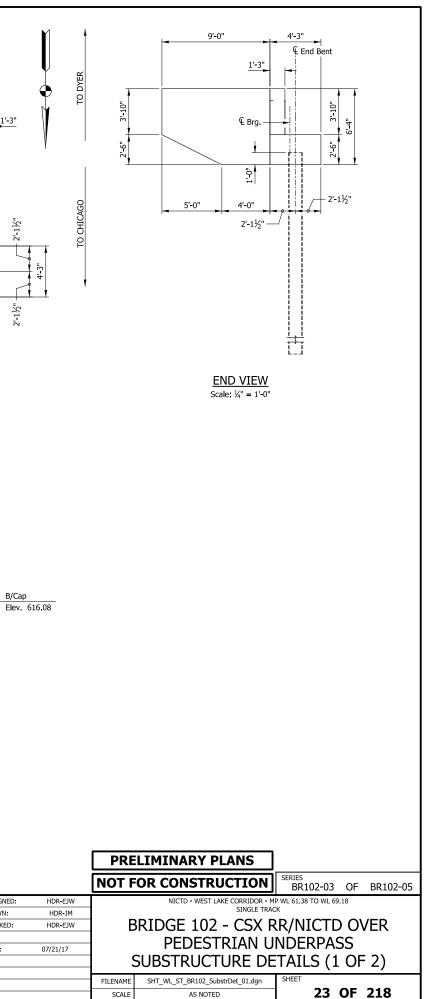
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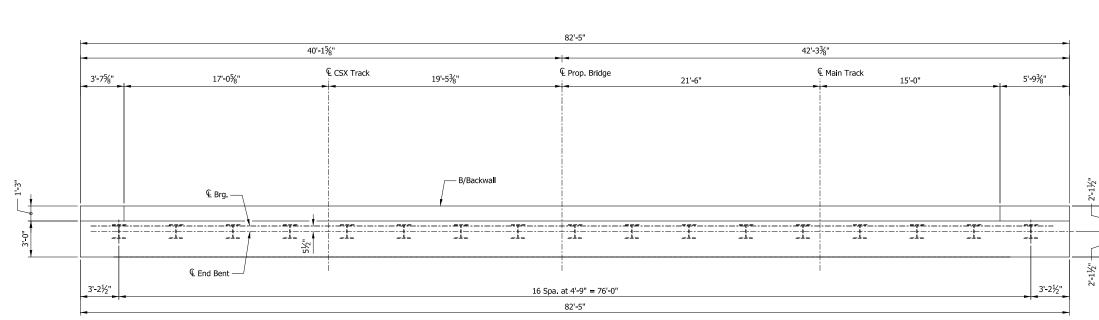


<u>PLAN</u> Scale: ¼" = 1'-0"

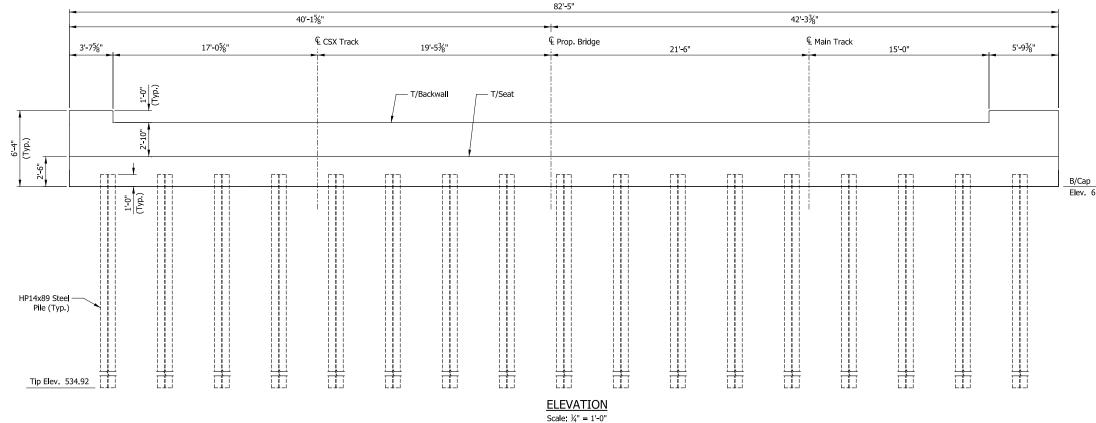


SOUTH END BENT DESIGNED: DRAWN: HECKED DATE: NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT 33 East Highway 12 Chesterton, Indiana 46304 RRIDOR 0 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631 www.hdrinc.com DYER TO HAMMOND, INDIANA ISSUE DATE DESCRIPTION





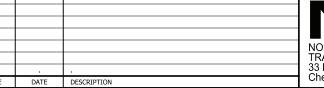
<u>PLAN</u> Scale: ¼" = 1'-0"



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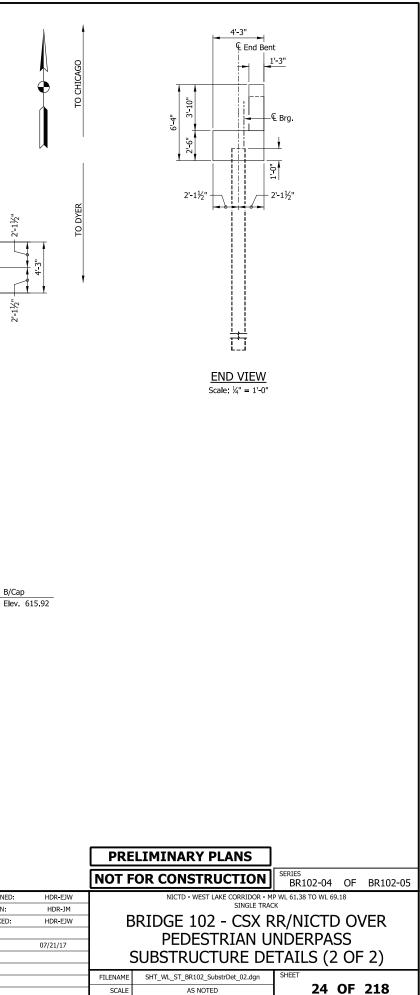
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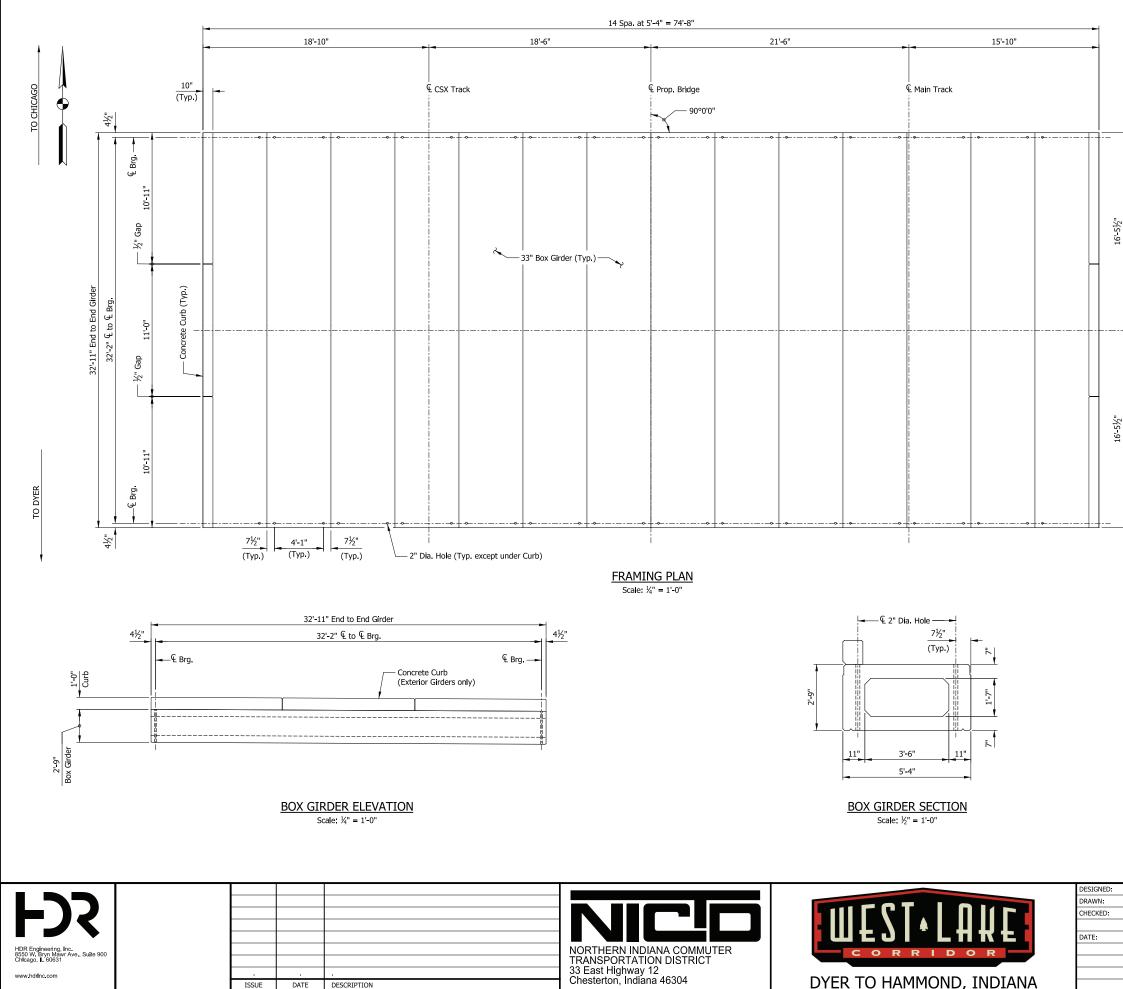




NORTH END BENT

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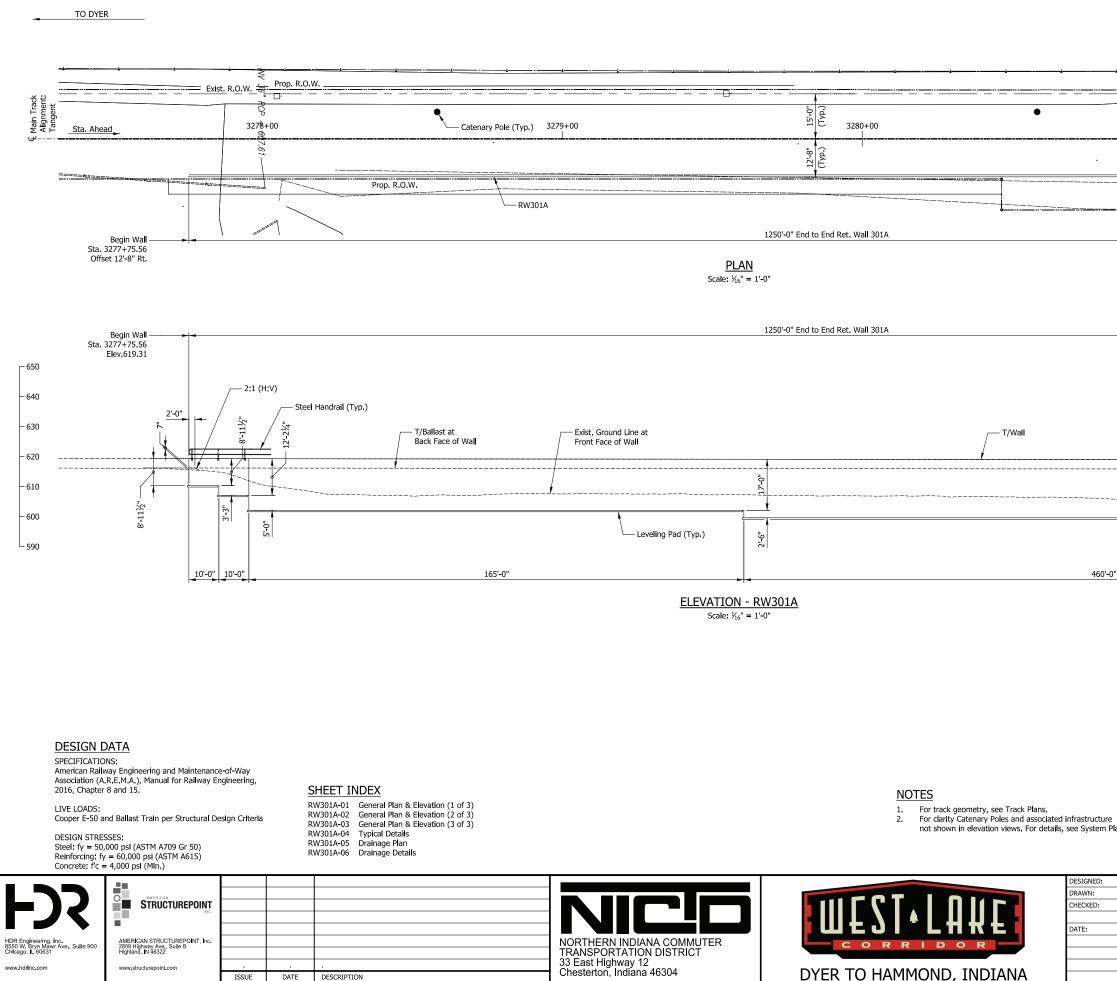
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	PRE	LIMINARY PLANS	
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HDR-EJW HDR-JM		NICTD - WEST LAKE CORRIDOR - M SINGLE TRAC	
HDR-JM HDR-EJW	BRIDGE 102 - CSX RR/NICTD OVER		
07/21/17	PEDESTRIAN UNDERPASS		
0//21/17	FRAMING PLAN		
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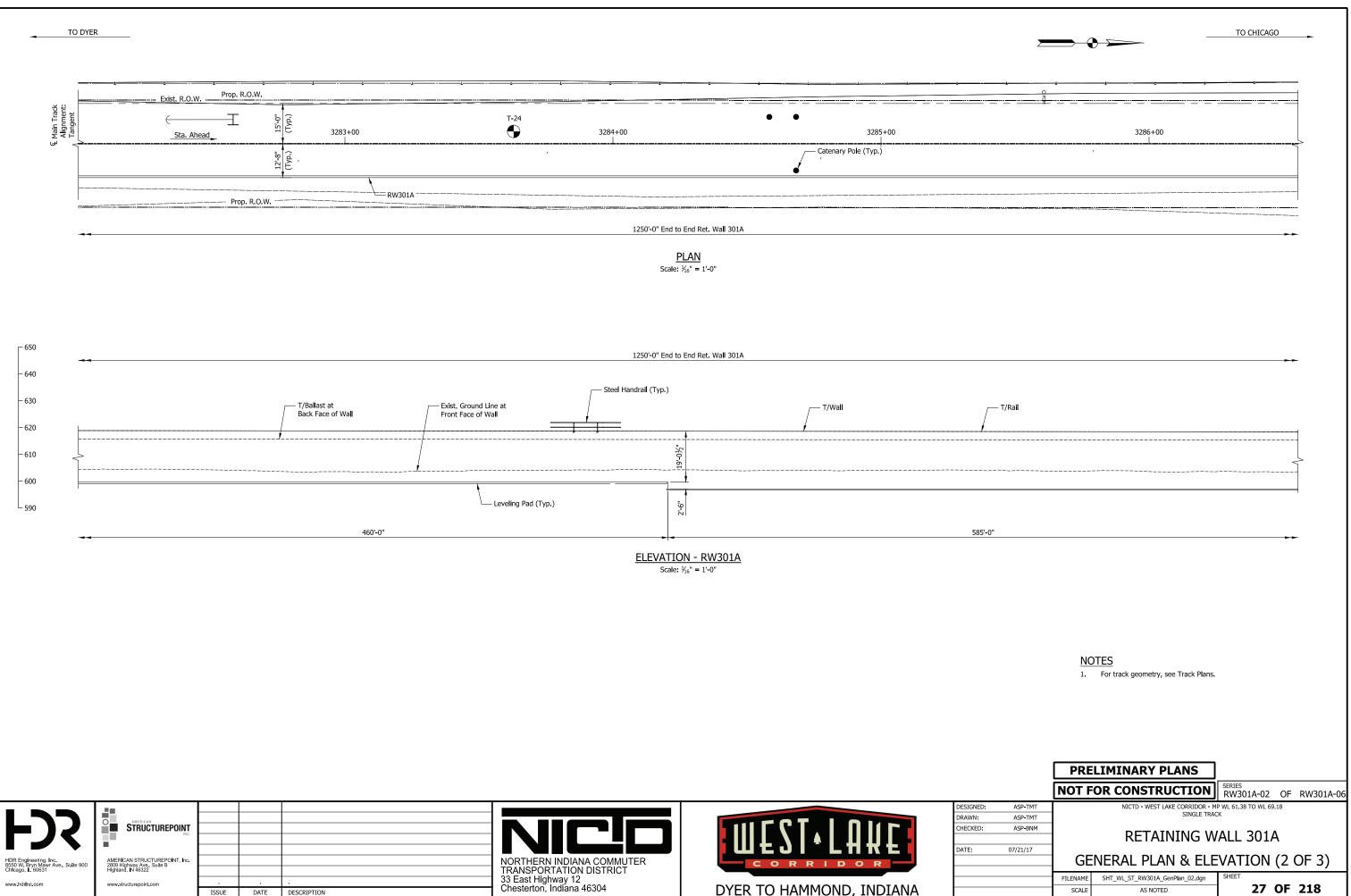
Pedestrian | Structure

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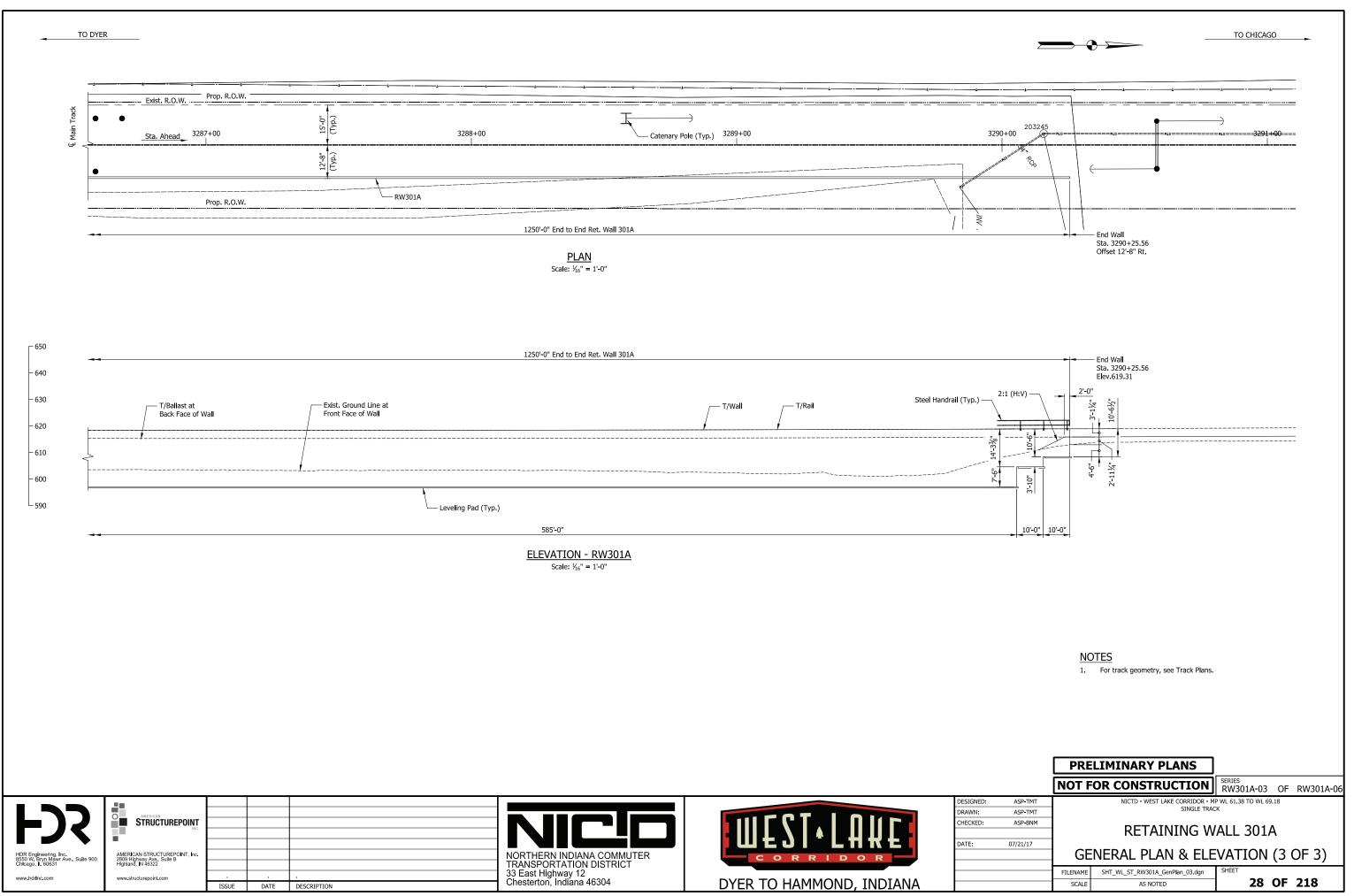


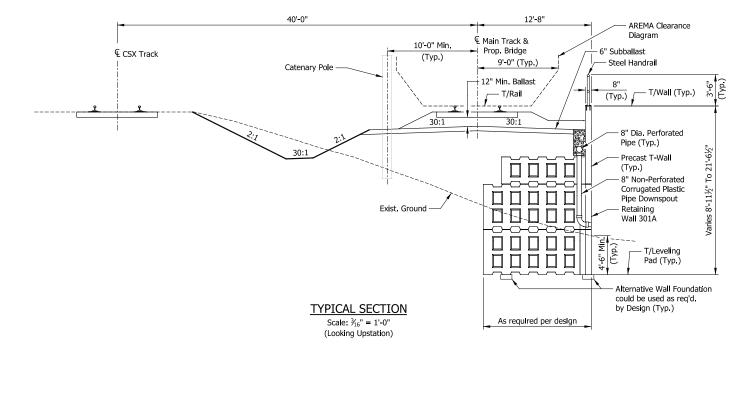
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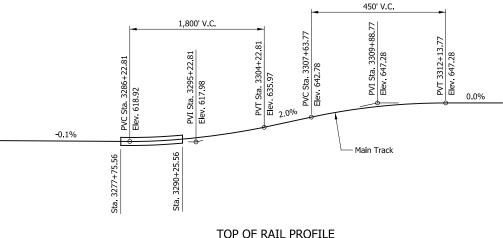
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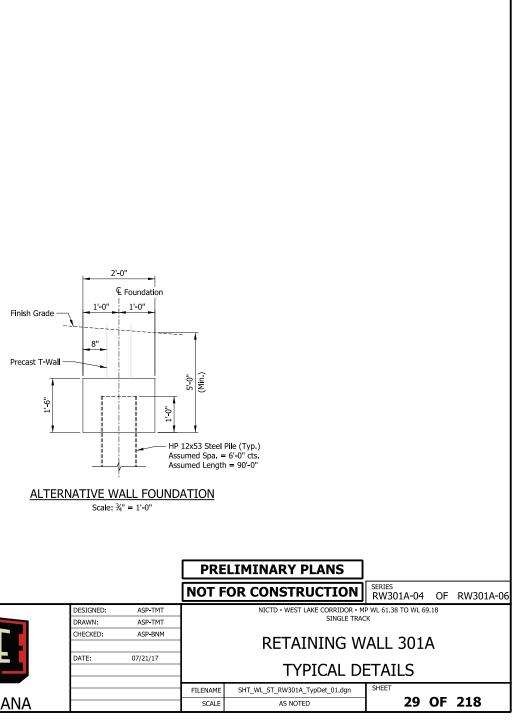


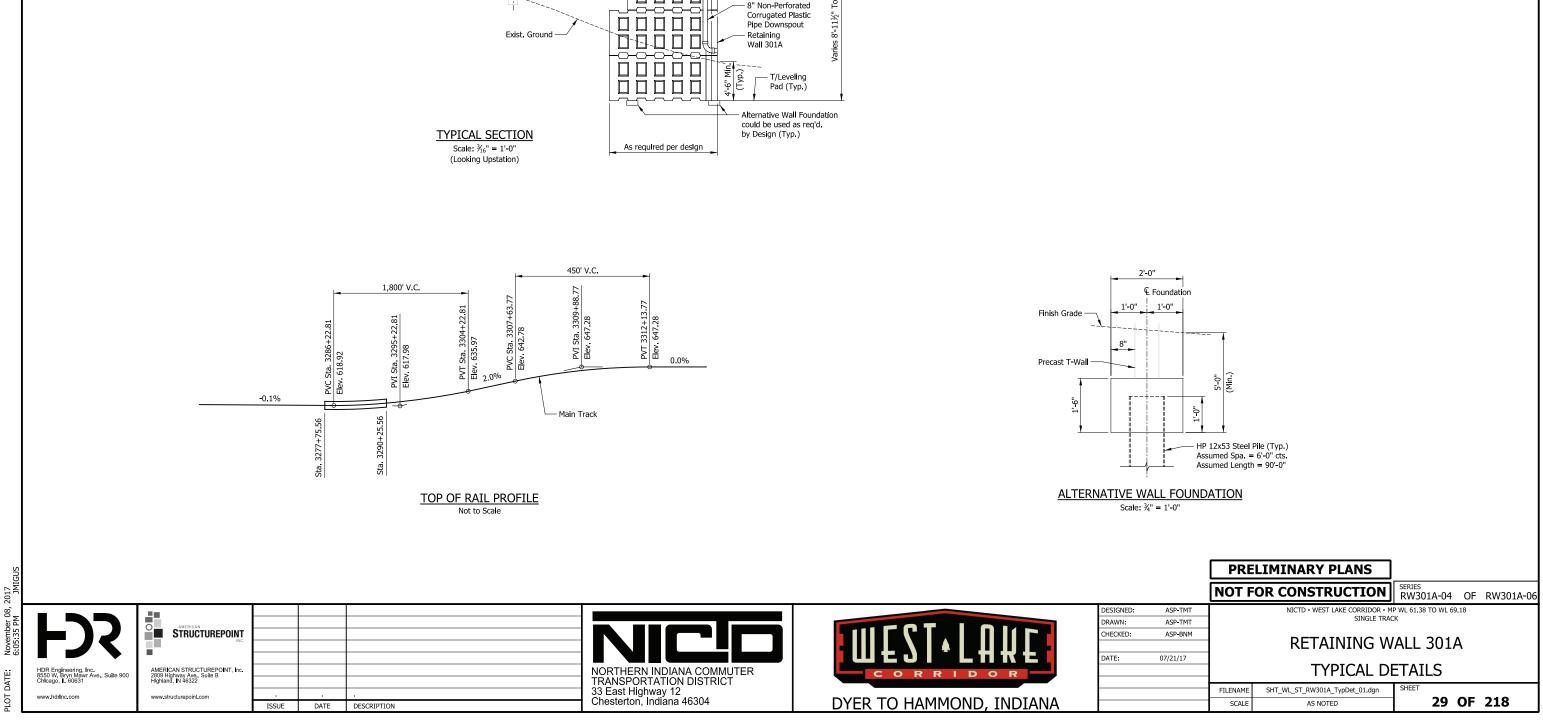


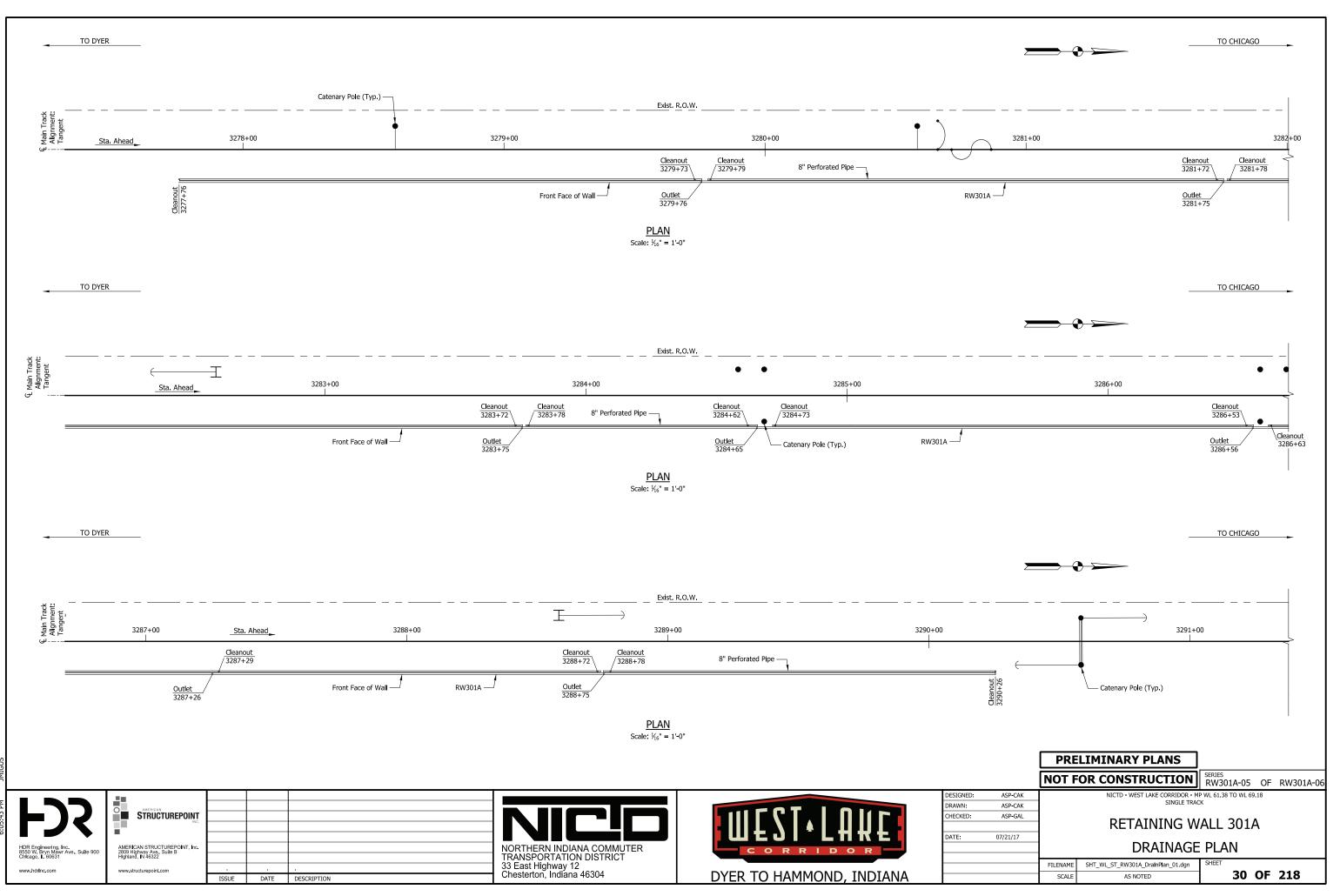




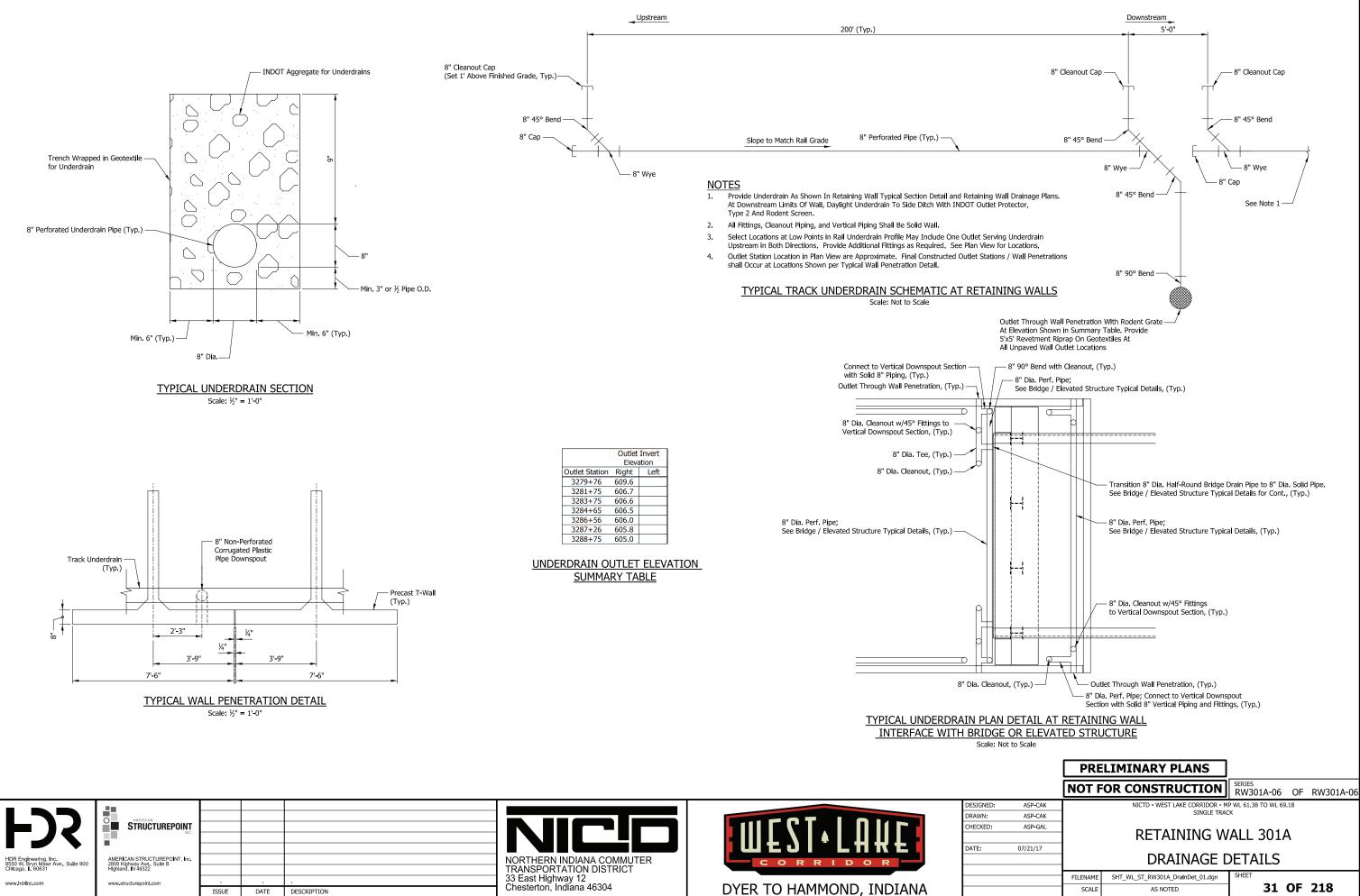






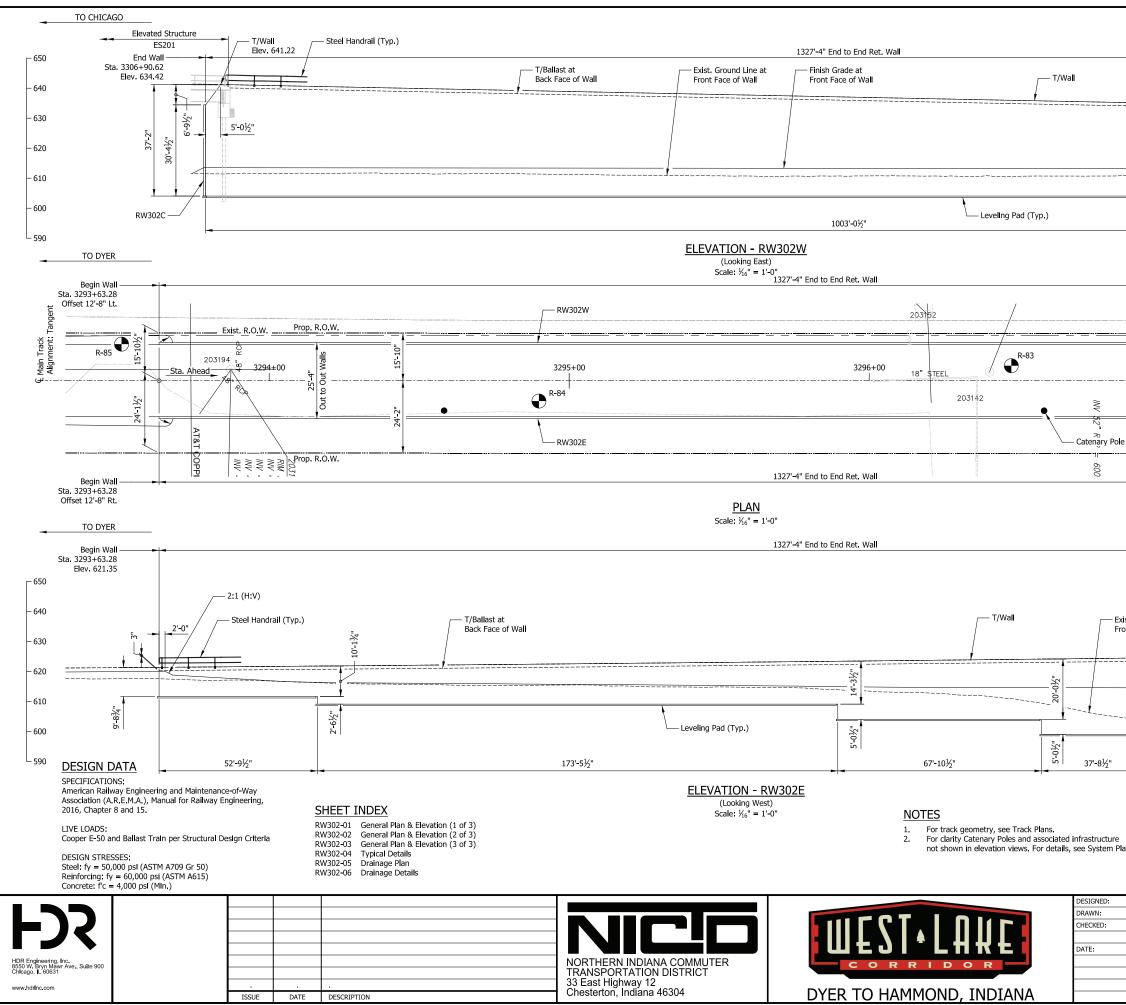


PLOT DATE: November 08, 2



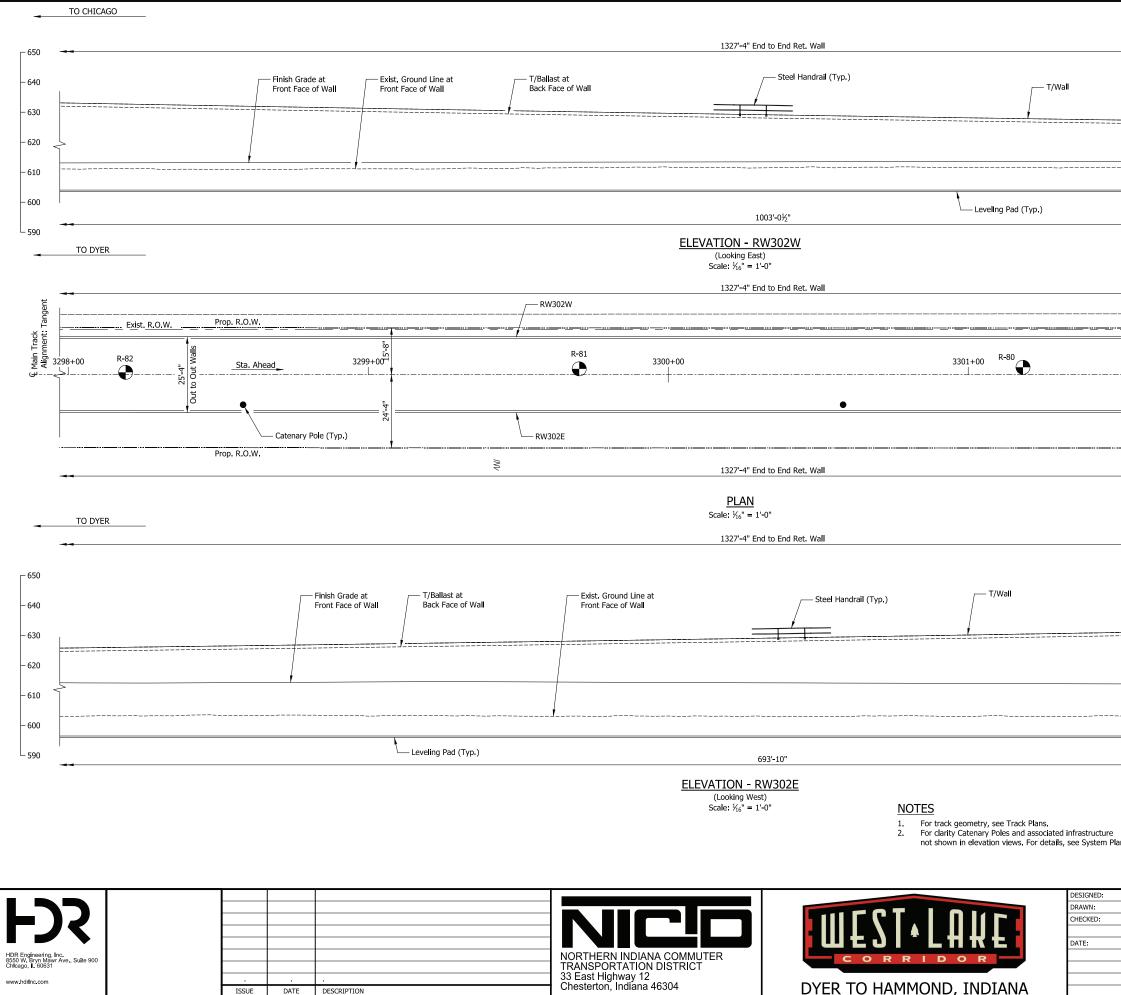
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gineering, Inc. Bryn Mawr Ave., Suite 900 , IL 60631	AMERICAN STRUCTUREPOINT, Inc. 2809 Highway Ave., Suite B Highland, IN 46322				NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT	
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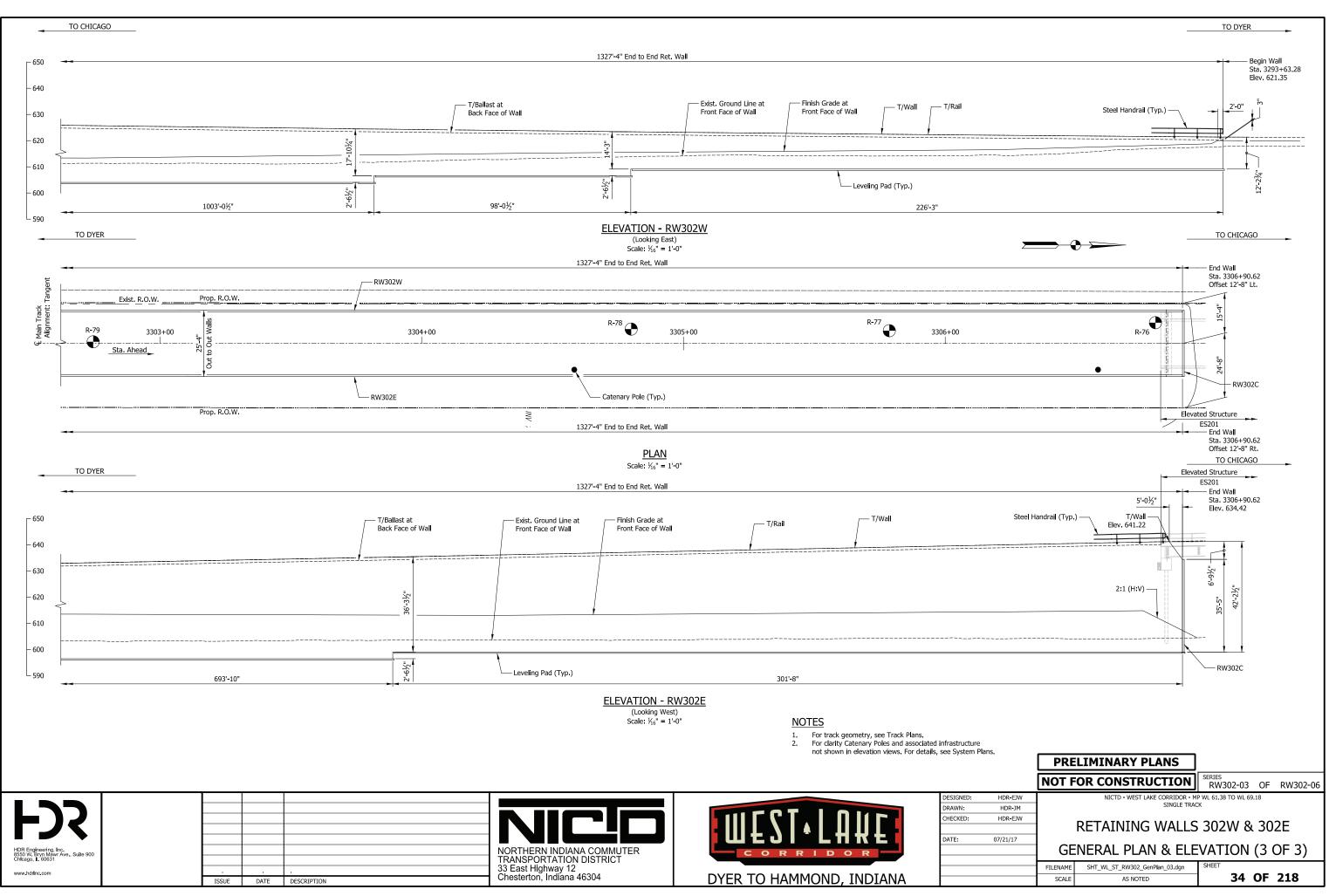


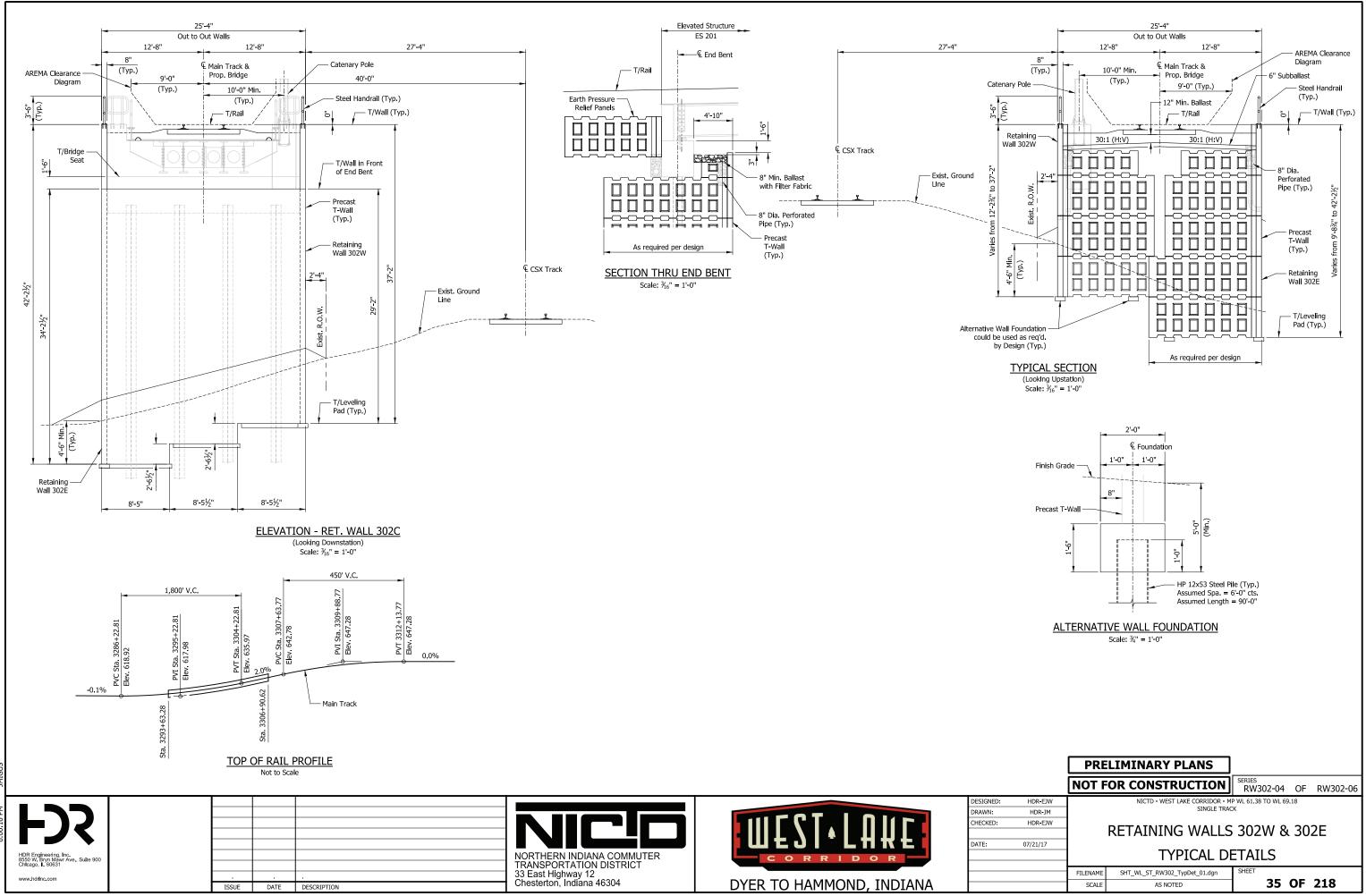
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25'-6¼"	
2'-6½"	693'-10"
e Plans.	
	PRELIMINARY PLANS
	NOT FOR CONSTRUCTION SERIES RW302-01 OF RW302-06
HDR-EJW	NICTD - WEST LAKE CORRIDOR - MP WL 61.38 TO WL 69.18 SINGLE TRACK
HDR-JM HDR-EJW	
07/21/17	RETAINING WALLS 302W & 302E
07/21/17	GENERAL PLAN & ELEVATION (1 OF 3)
	FILENAME SHT_WL_ST_RW302_GenPlan_01.dgn SHEET
	SCALE AS NOTED 32 OF 218

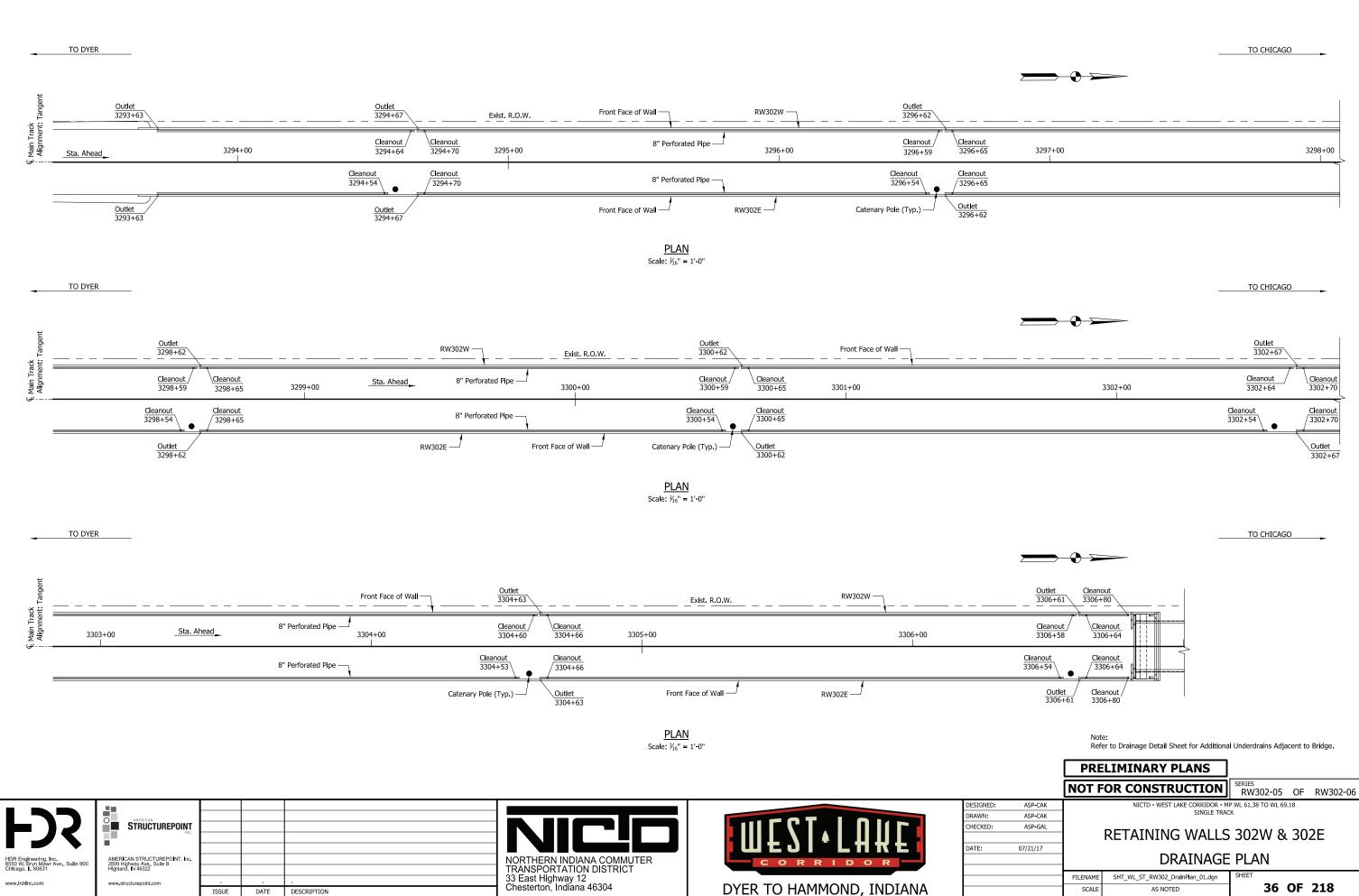


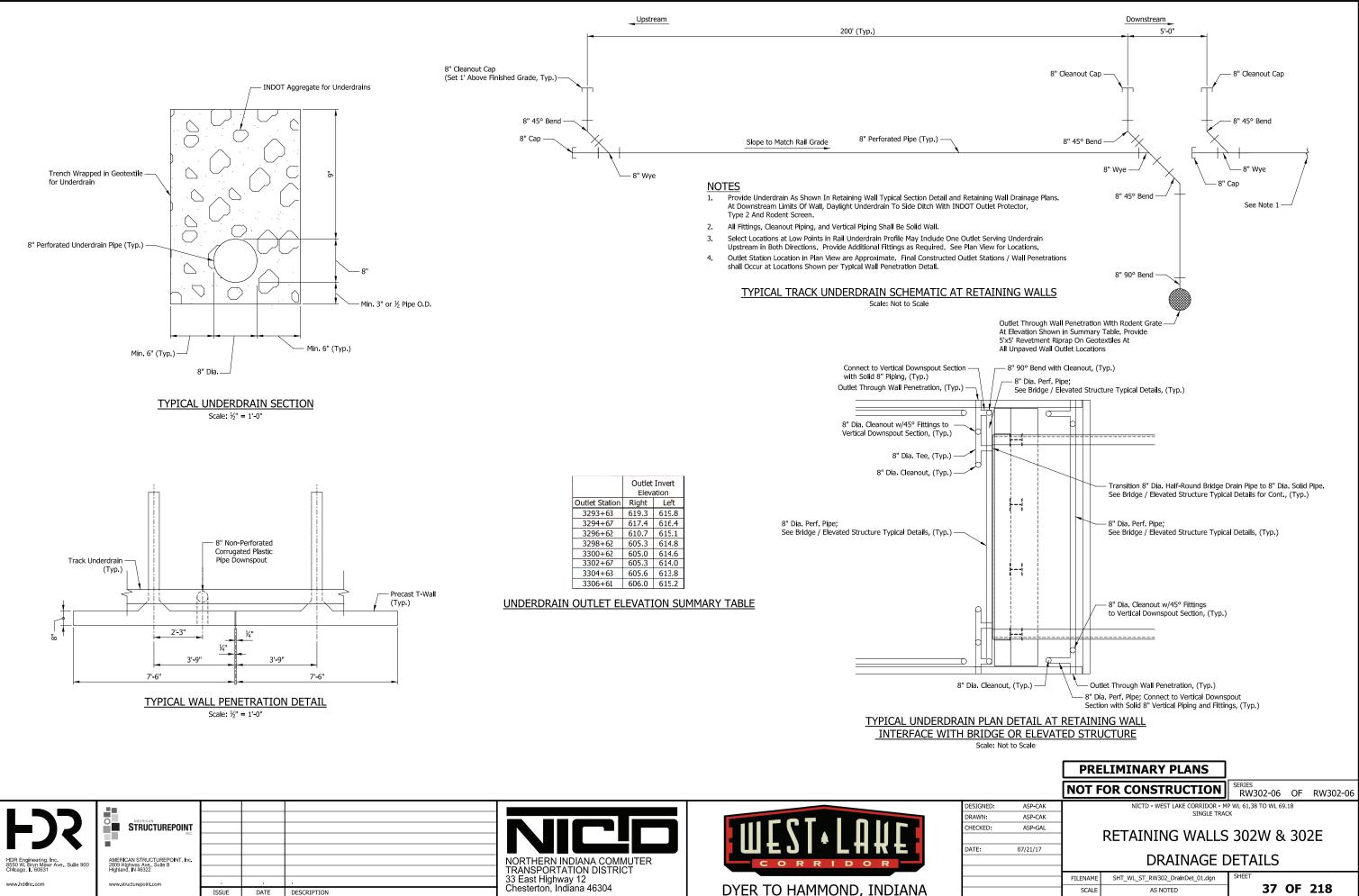
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Plans.	PRELIMINARY PLANS	
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	NOT FOR CONSTRUCTION	RW302-02 OF RW302-06
HDR-EJW HDR-JM	NICTD - WEST LAKE CORRIDOR - MF SINGLE TRAC	
HDR-EJW		202111 0. 2025
07/21/17	RETAINING WALLS	
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	FILENAME SHT_WL_ST_RW302_GenPlan_02.dgn	SHEET
	SCALE AS NOTED	33 OF 218





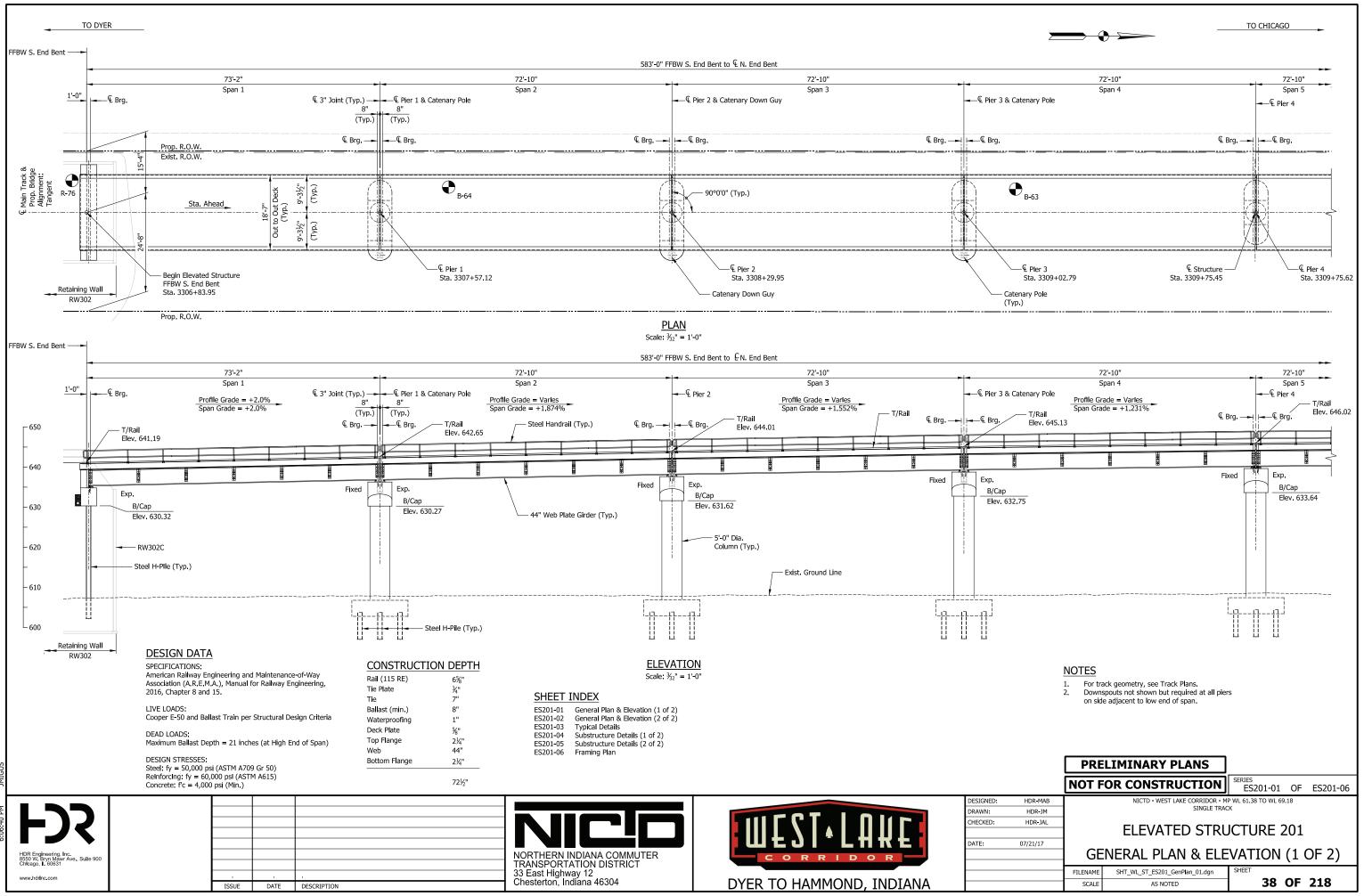
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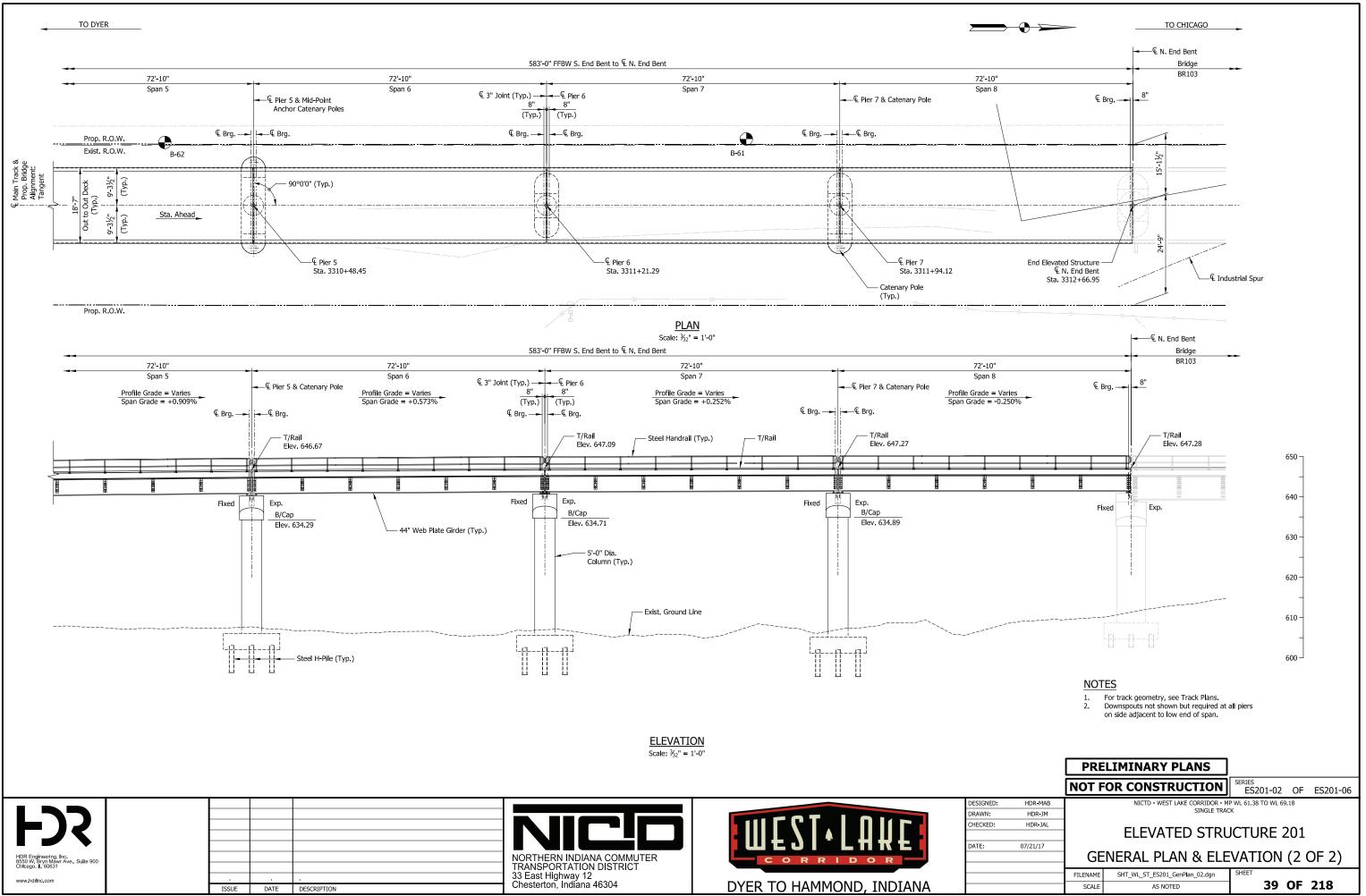


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neering, Inc. Bryn Mawr Ave., Suite 900 IL 60631	AMERICAN STRUCTUREPOINT, Inc. 2809 Highway Ave., Suite B Highland, IN 46322				NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT	
nc.com	www.structurepoint.com	ISSUE	DATE	DESCRIPTION	33 East Highway 12 Chesterton, Indiana 46304	DYE

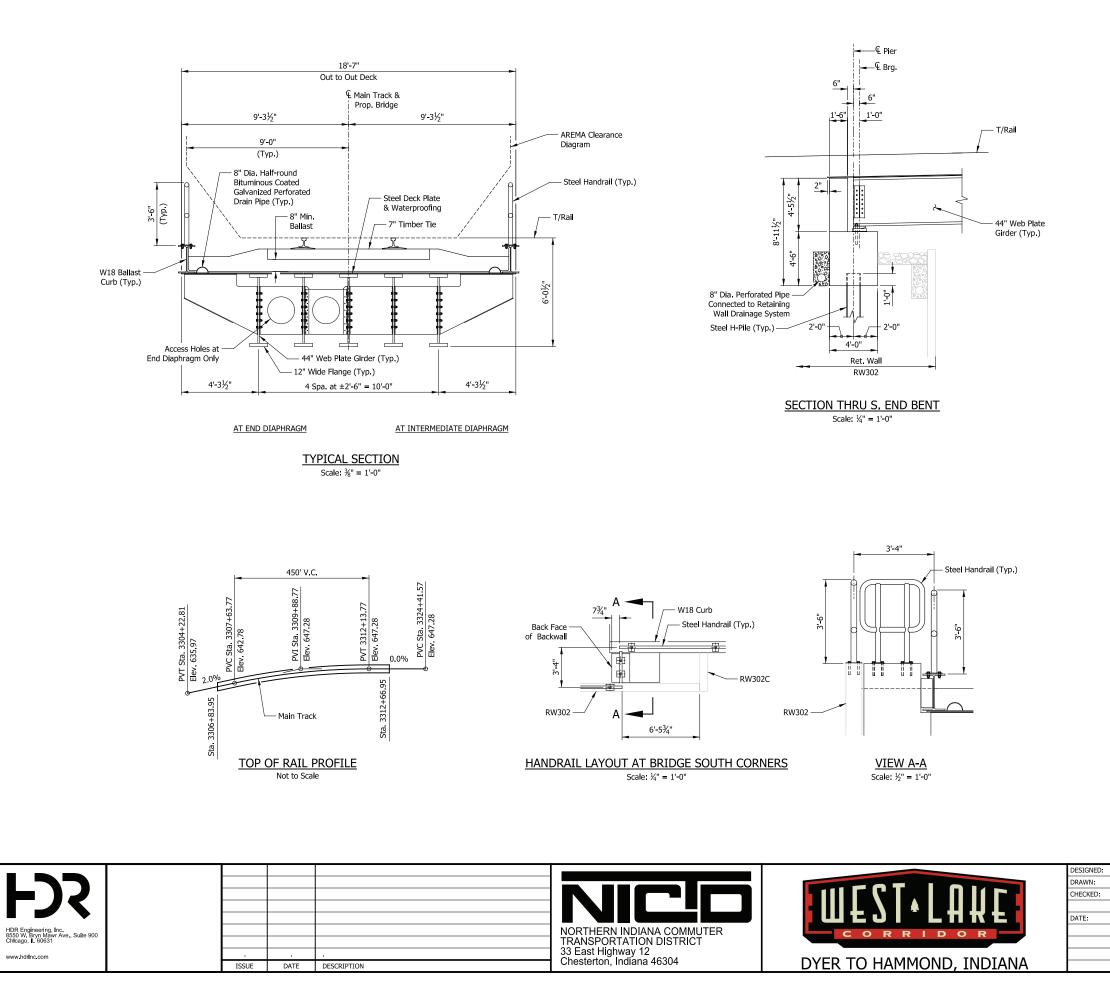




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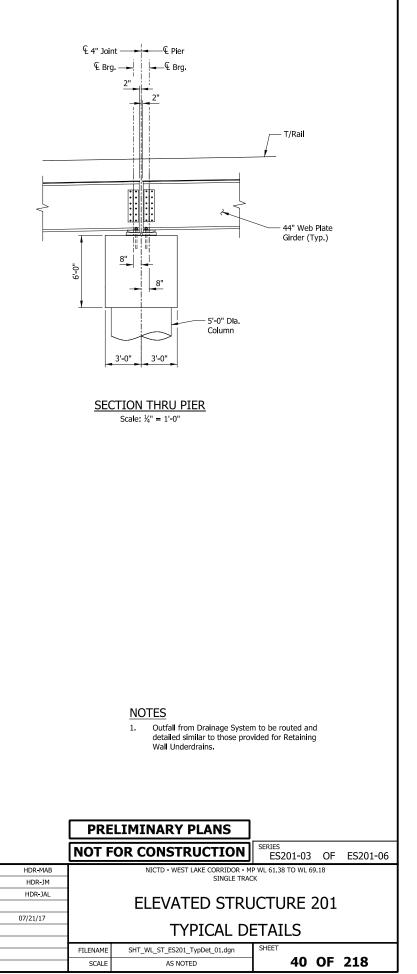


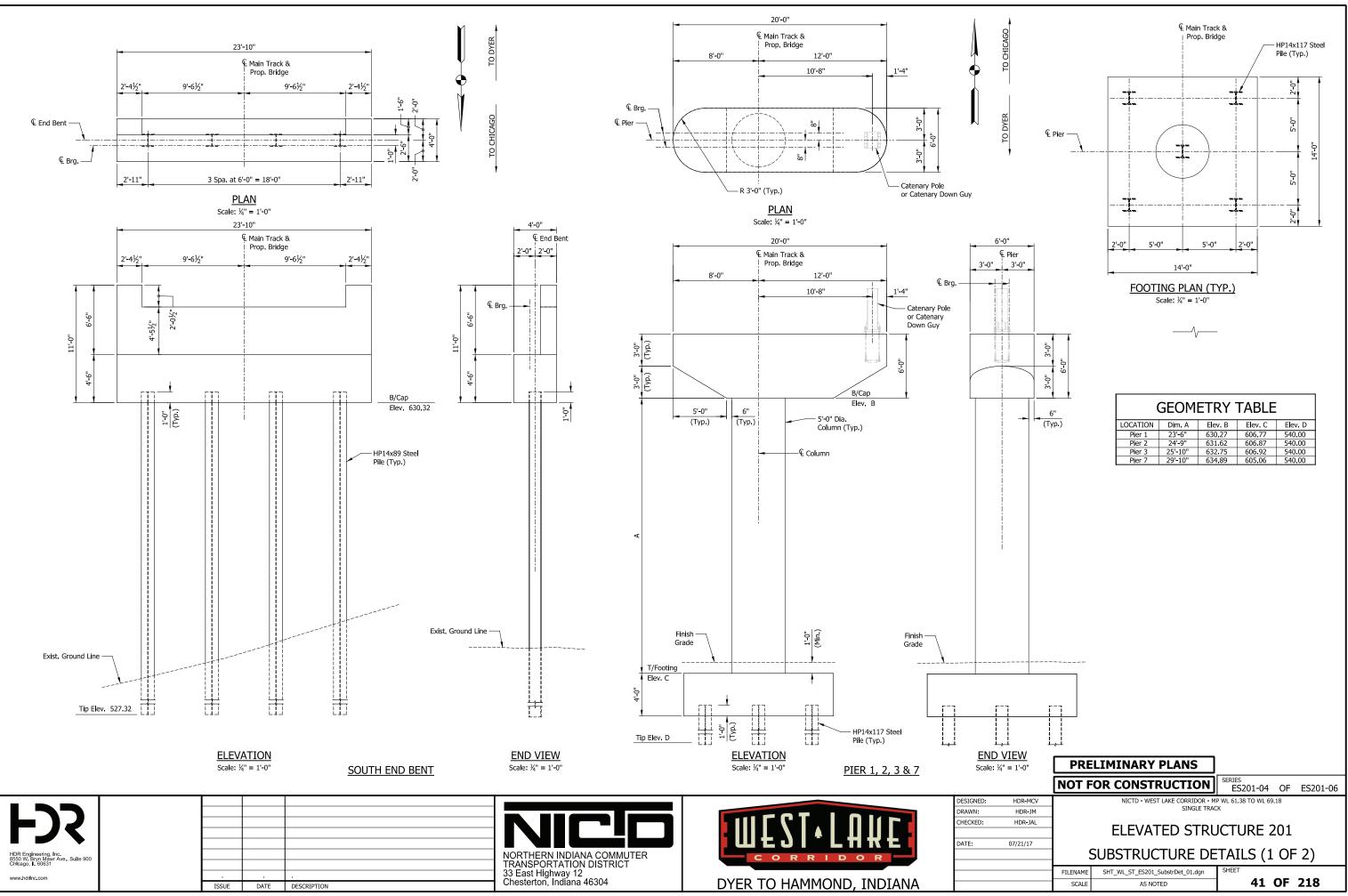
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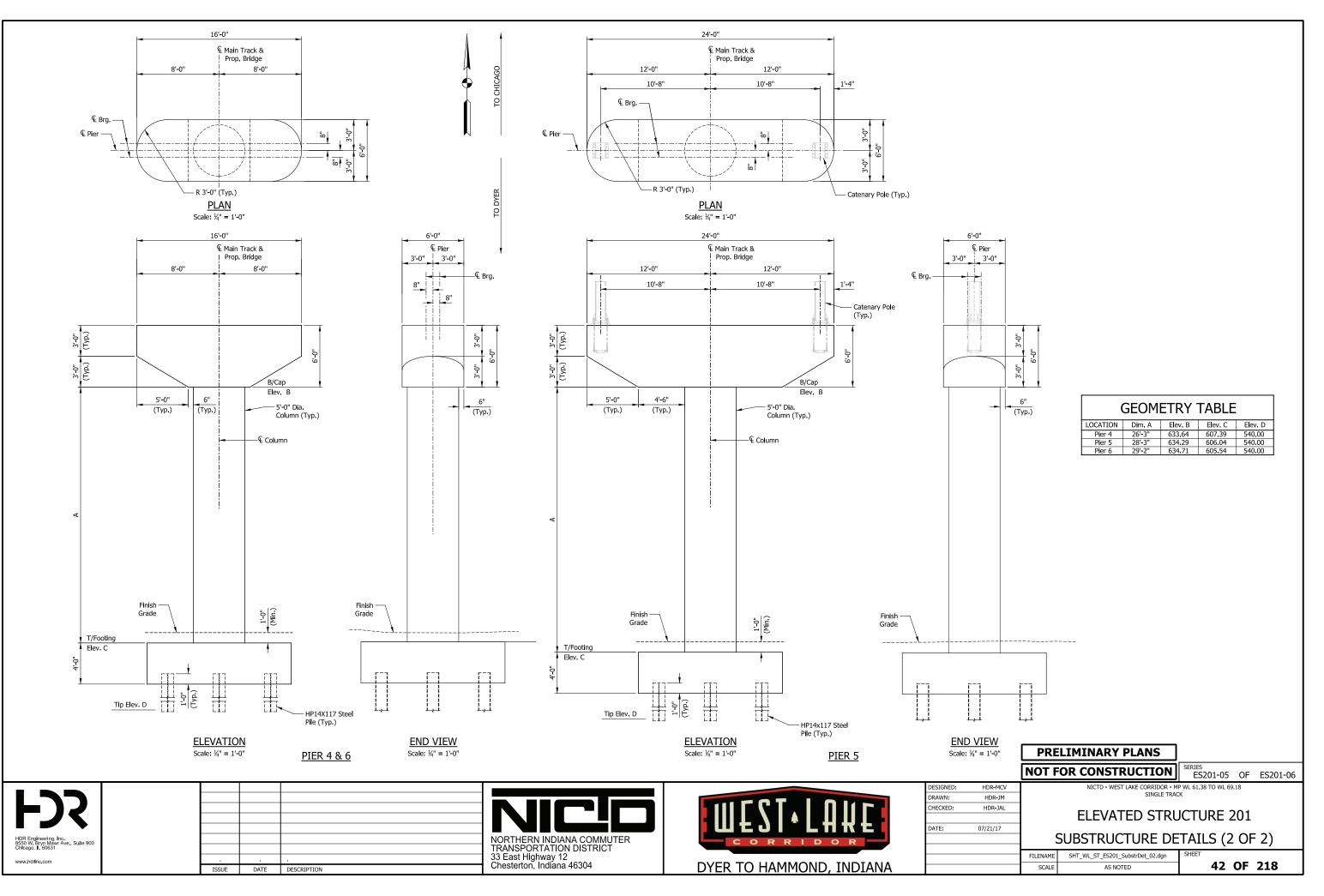


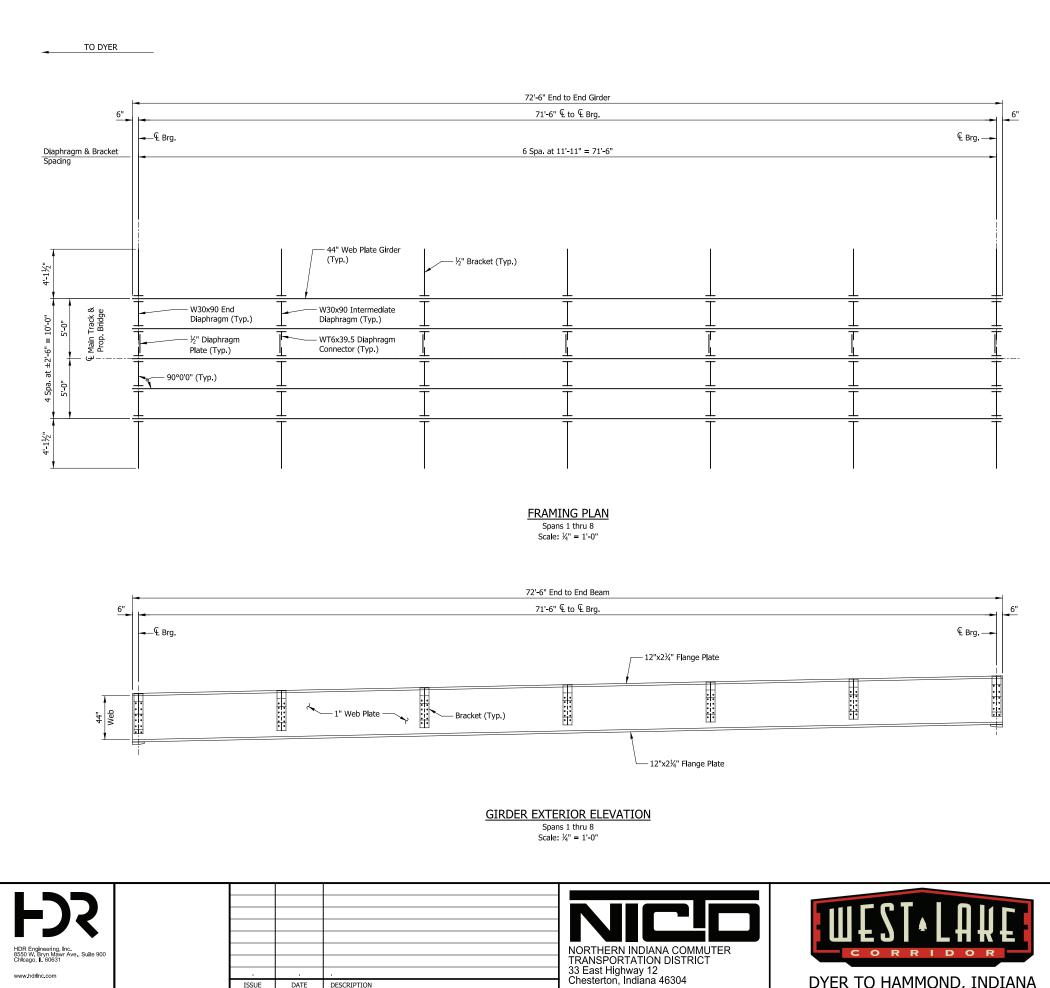
PLOT DATE: November 08, 2017 6.06.53 PM IMIGUS

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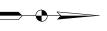


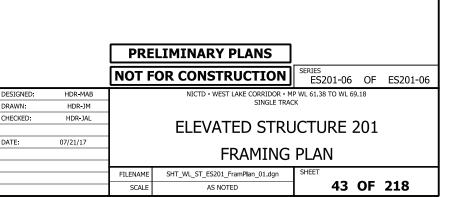
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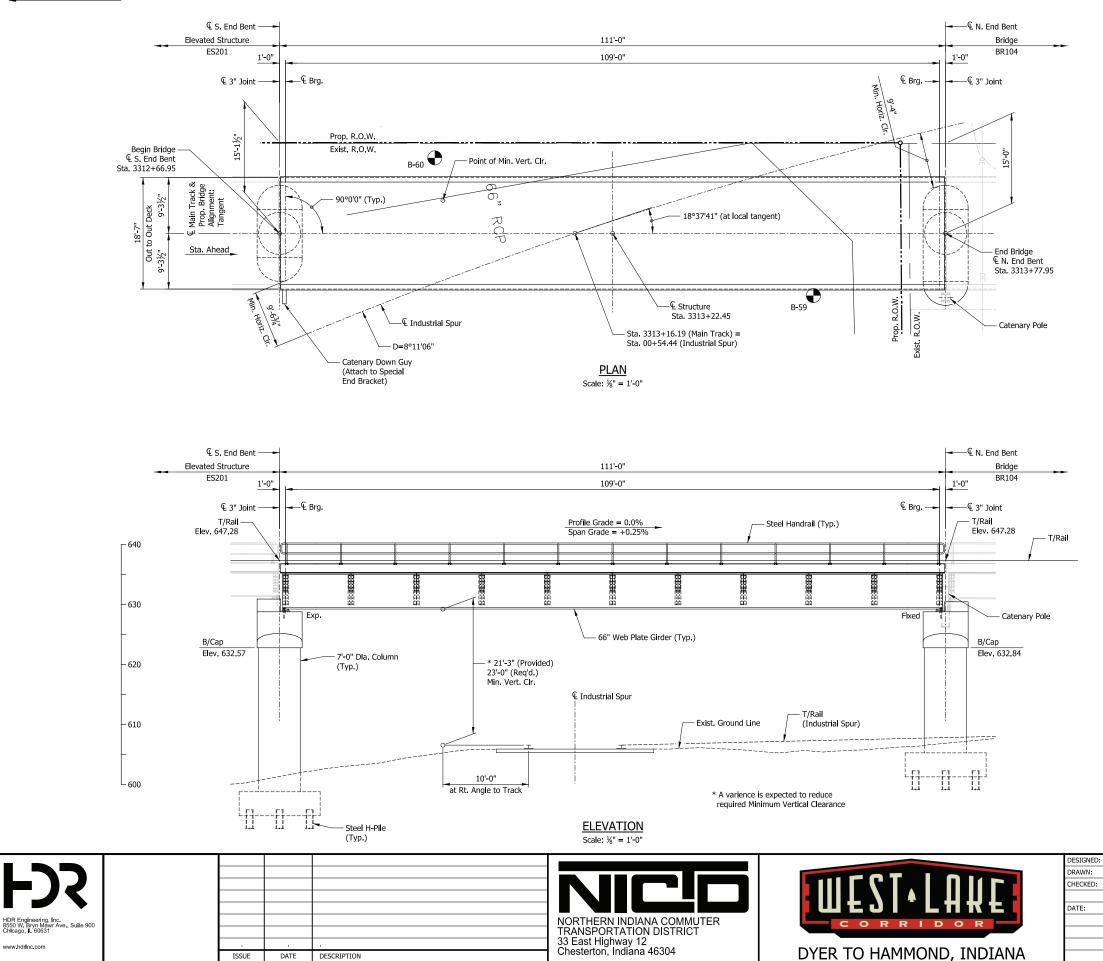
DYER TO HAMMOND, INDIANA

TO CHICAGO





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PLOT DATE: November 08, 2017 6-07-40 PM IMIGUS TO CHICAGO

DESIGN DATA

SPECIFICATIONS: American Railway Engineering and Maintenance-of-Way Association (A.R.E.M.A.), Manual for Railway Engineering, 2016, Chapter 8 and 15.

LIVE LOADS: Cooper E-50 and Ballast Train per Structural Design Criteria

DEAD LOADS: Maximum Ballast Depth = 21 inches (at High End of Span)

DESIGN STRESSES: Steel: fy = 50,000 psi (ASTM A709 Gr 50) Reinforcing: fy = 60,000 psi (ASTM A615) Concrete: f'c = 4,000 psi (Min.)

SHEET INDEX

BR103-01	General Plan & Elevation
BR103-02	Typical Details
BR103-03	Substructure Details
BR103-04	Framing Plan

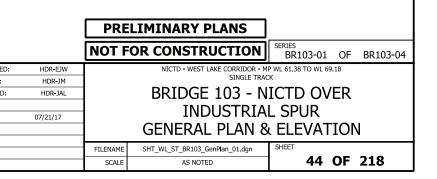
CONSTRUCTION DEPTH

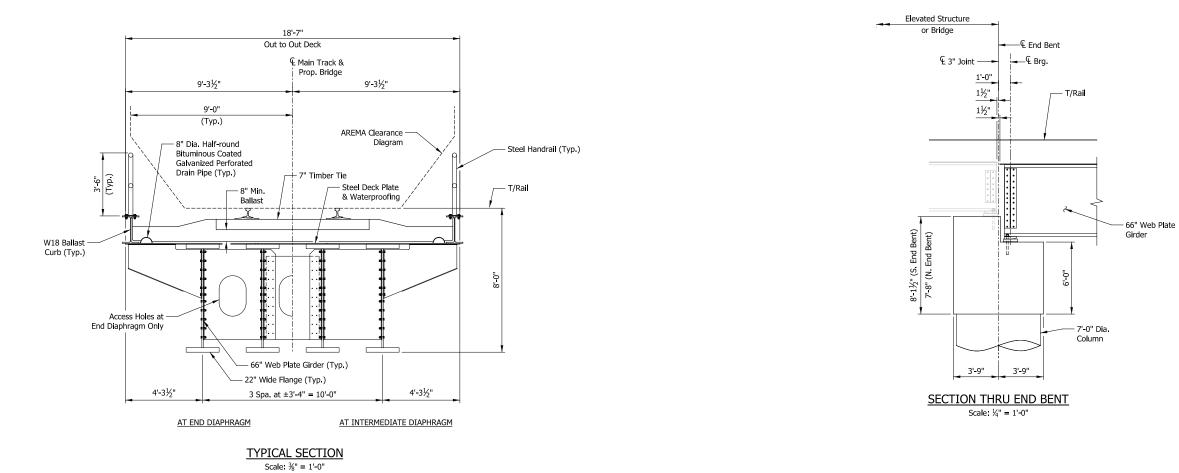
Rail (115 Lbs)	6%"
Tie Plate	34"
Tie	7"
Ballast (min.)	8"
Waterproofing	1"
Deck Plate	%"
Top Flange	3"
Web	66"
Bottom Flange	3"

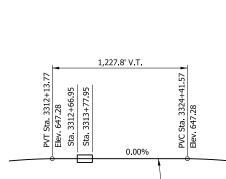
96"

NOTES

- For track geometry, see Track Plans.
 Downspouts not shown but required at all bents
- Downspouts not shown but required at all bents on side adjacent to low end of span.







Main Track

TOP OF RAIL PROFILE Not to Scale

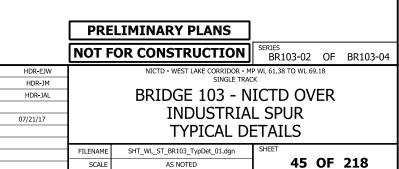
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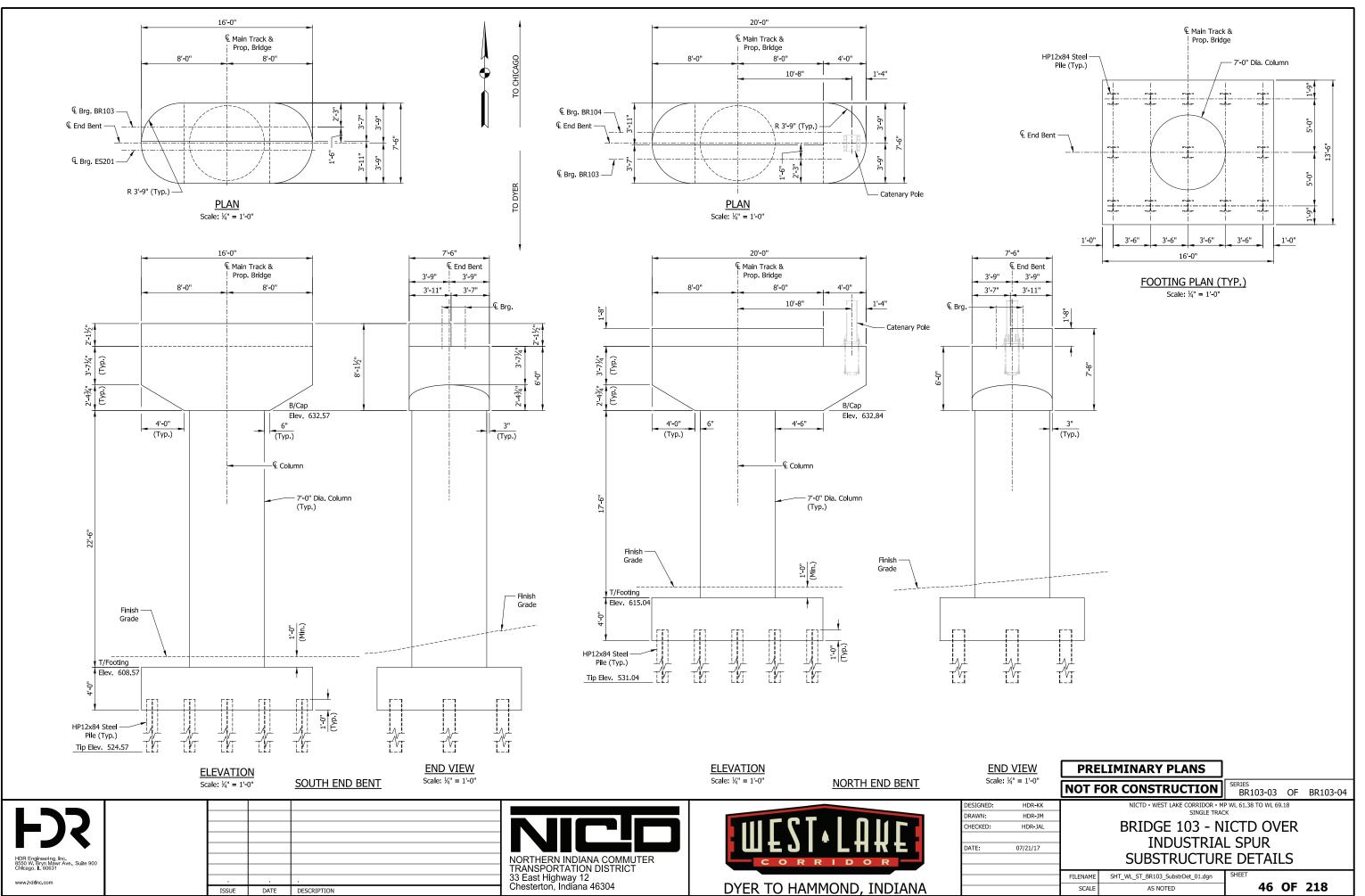
HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631

www.hdrinc.com

			NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT 33 East Highway 12 Chesterton, Indiana 46304	
ISSUE	DATE	DESCRIPTION	Chesterton, Indiana 40304	DYER TO HAMMOND, INDIANA

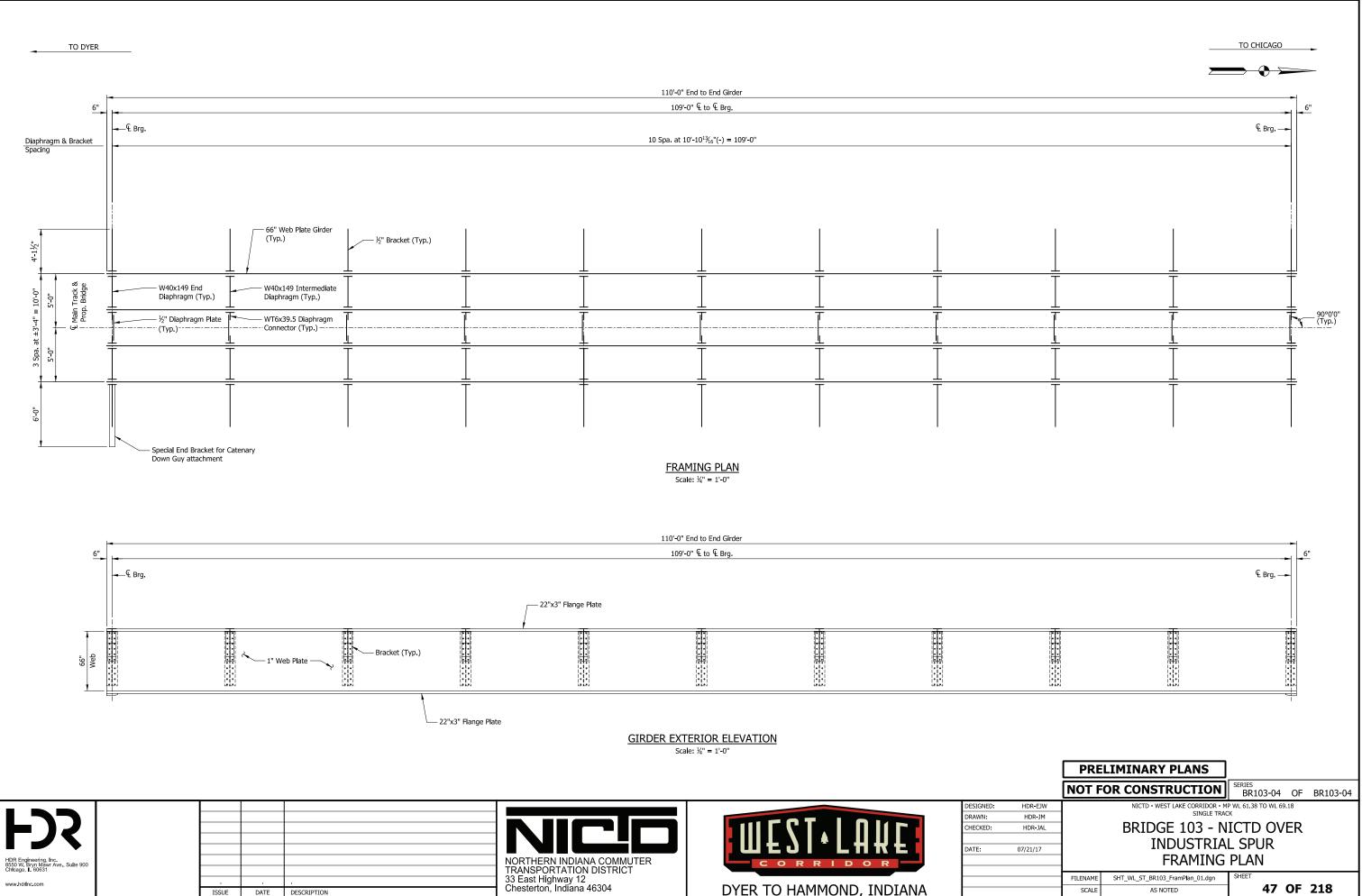
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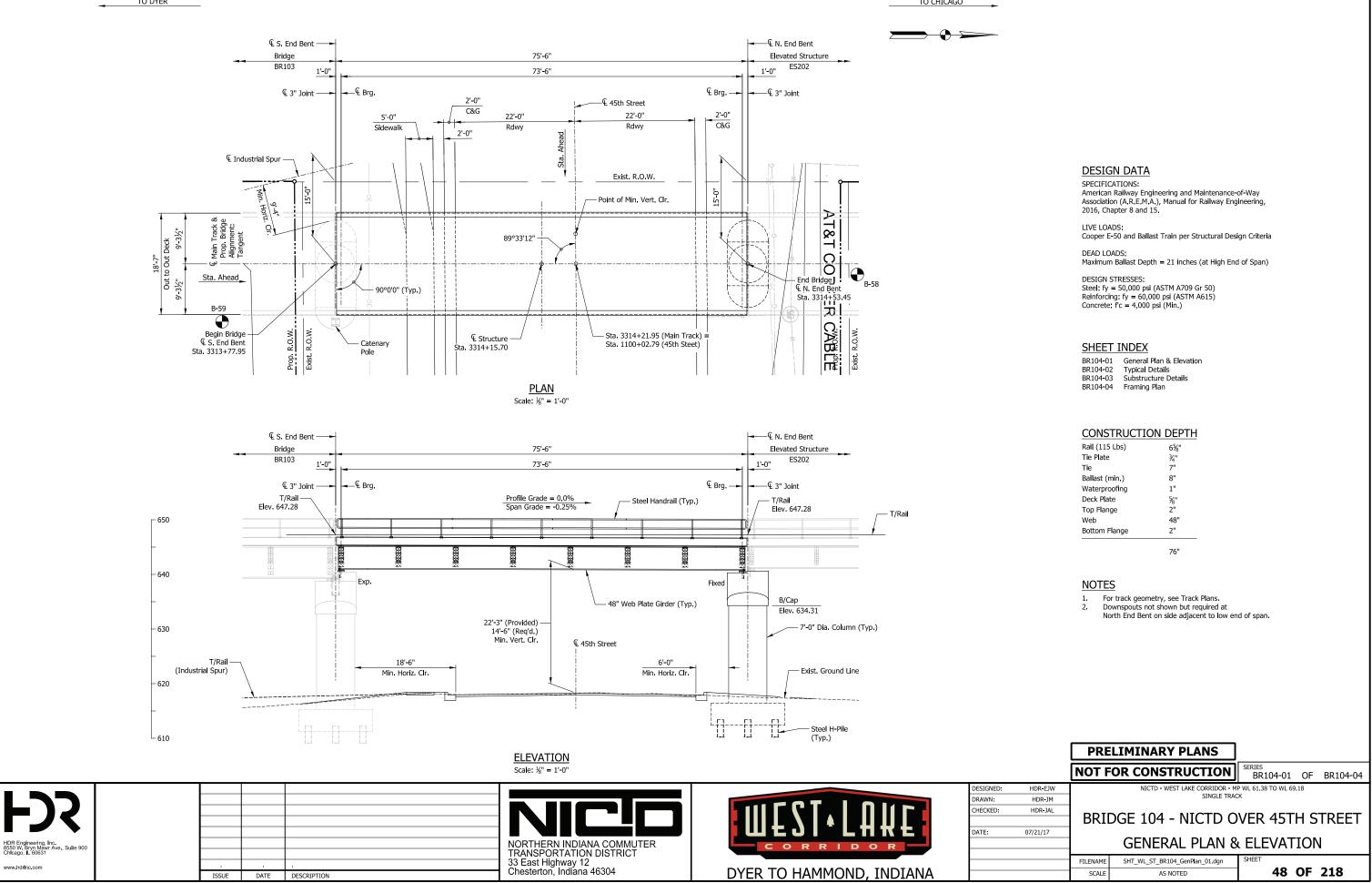
DYER TO HAMMOND, INDIANA



DATE

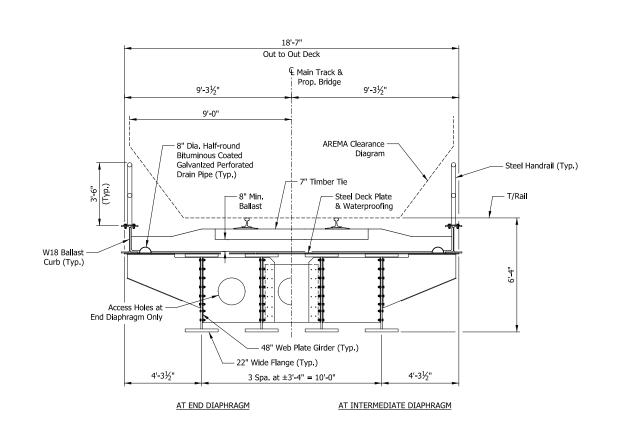
PLOT

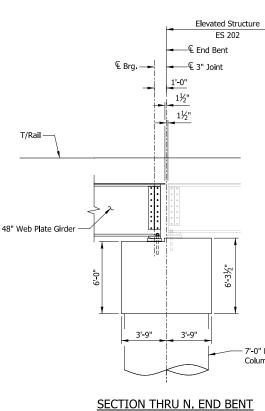
TO CHICAGO



BR104-01	General Plan & Elevation
BR104-02	Typical Details
BR104-03	Substructure Details
BR104-04	Framing Plan

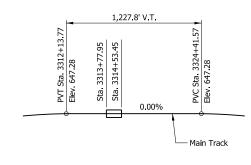
Rail (115 Lbs)	6%"
Tie Plate	3⁄4"
Tie	7"
Ballast (min.)	8"
Waterproofing	1"
Deck Plate	%"
Top Flange	2"
Web	48"
Bottom Flange	2"





TYPICAL SECTION

Scale: 3/8" = 1'-0"



TOP OF RAIL PROFILE Not to Scale

DATE PLOT

HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631

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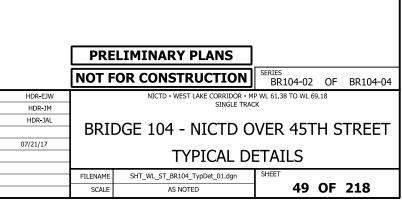
			NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT	CO
			33 East Highway 12 Chesterton, Indiana 46304	
ISSUE	DATE	DESCRIPTION	Chesterton, Indiana 46304	DYER TC

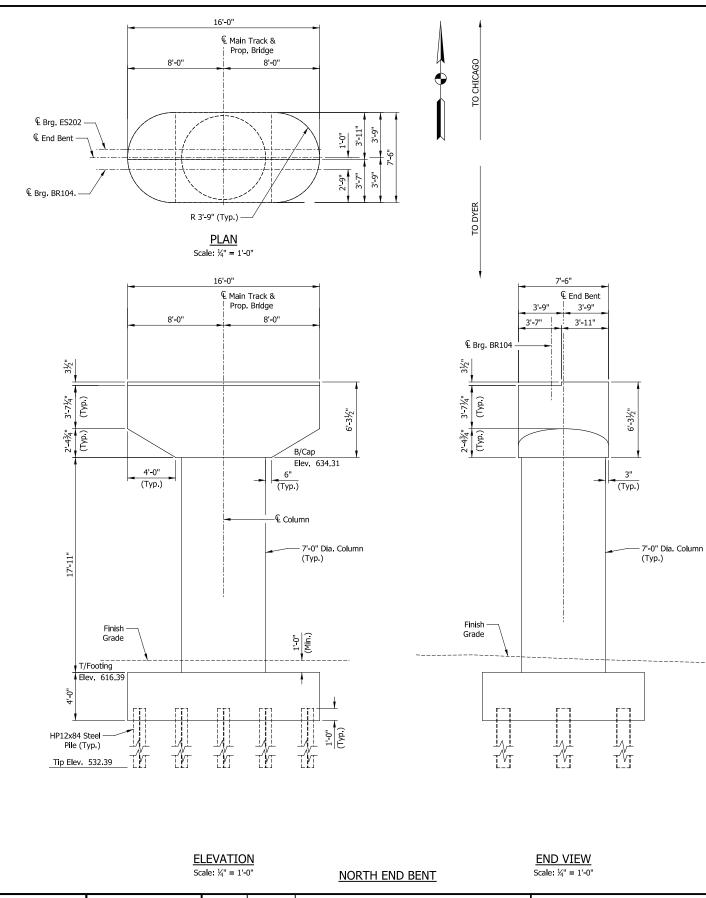


DESIGNED: DRAWN: CHECKED: DATE:

– 7'-0" Dia. Column

Scale: ¼" = 1'-0"





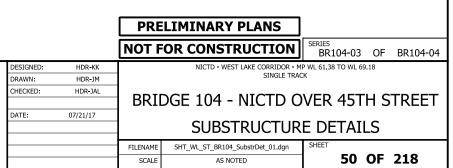


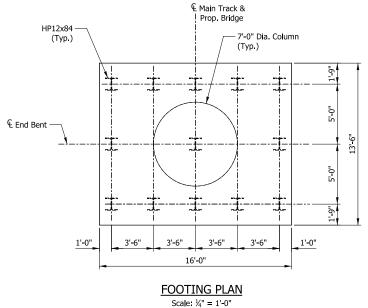
8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631 www.hdrinc.com

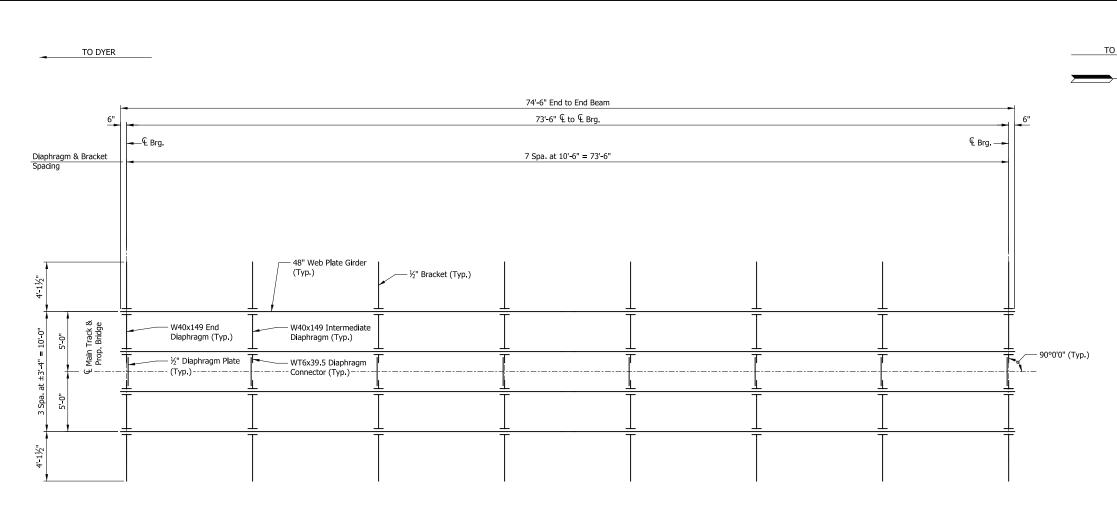
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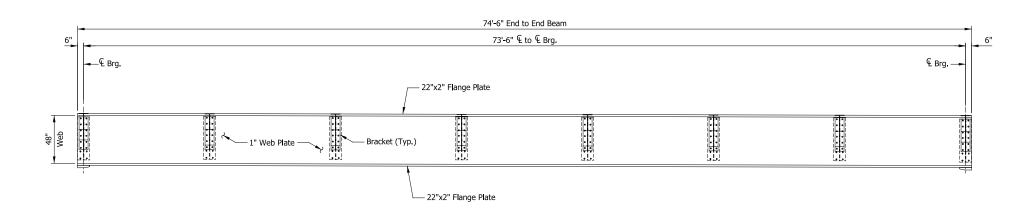




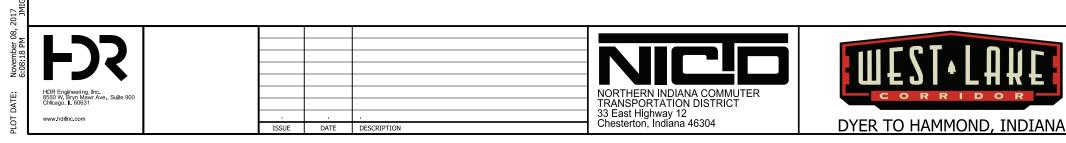




FRAMING PLAN Scale: ½" = 1'-0"

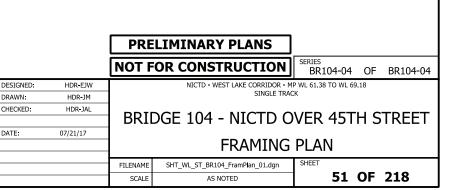


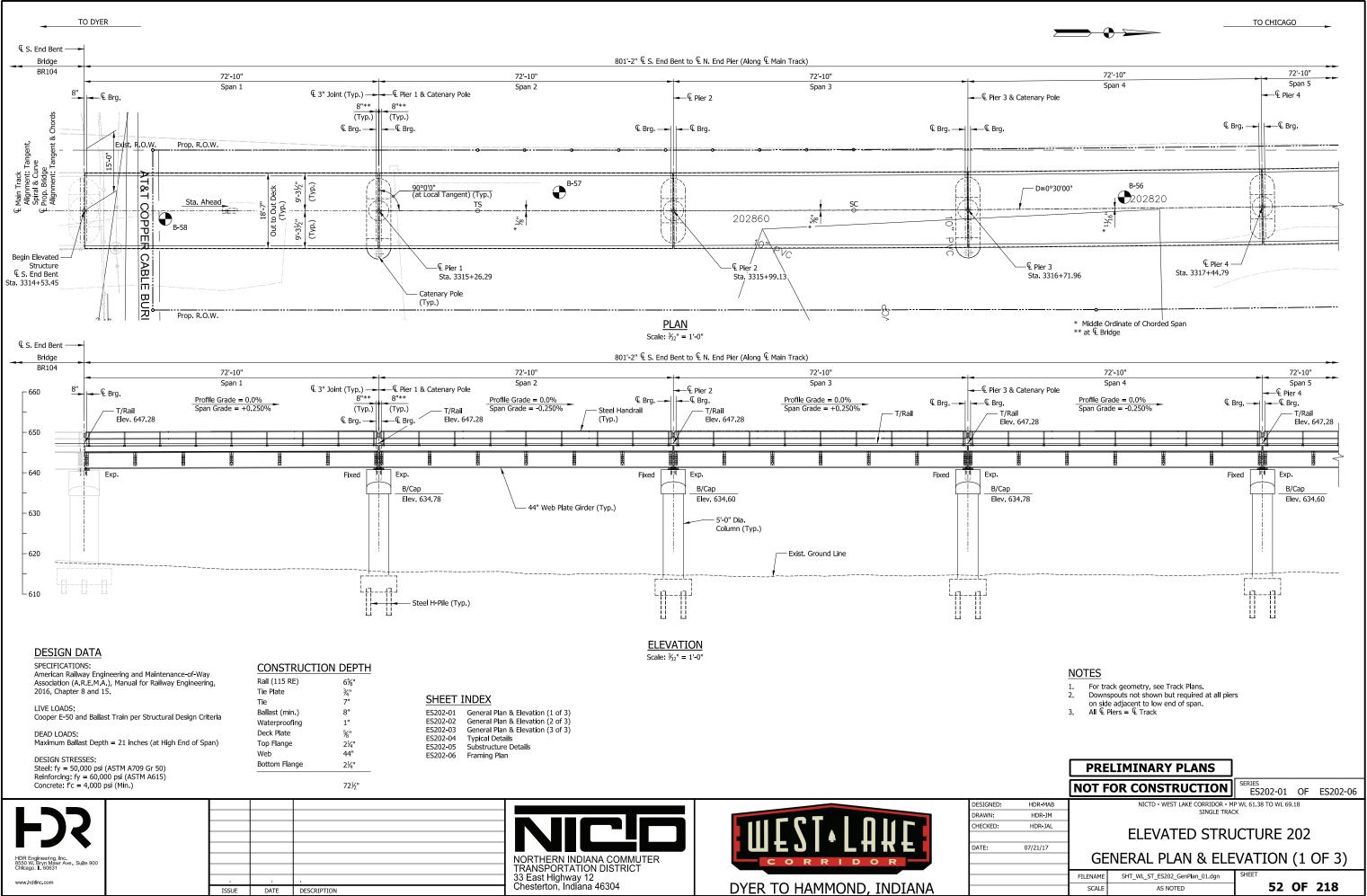
GIRDER EXTERIOR ELEVATION Scale: ½" = 1'-0"



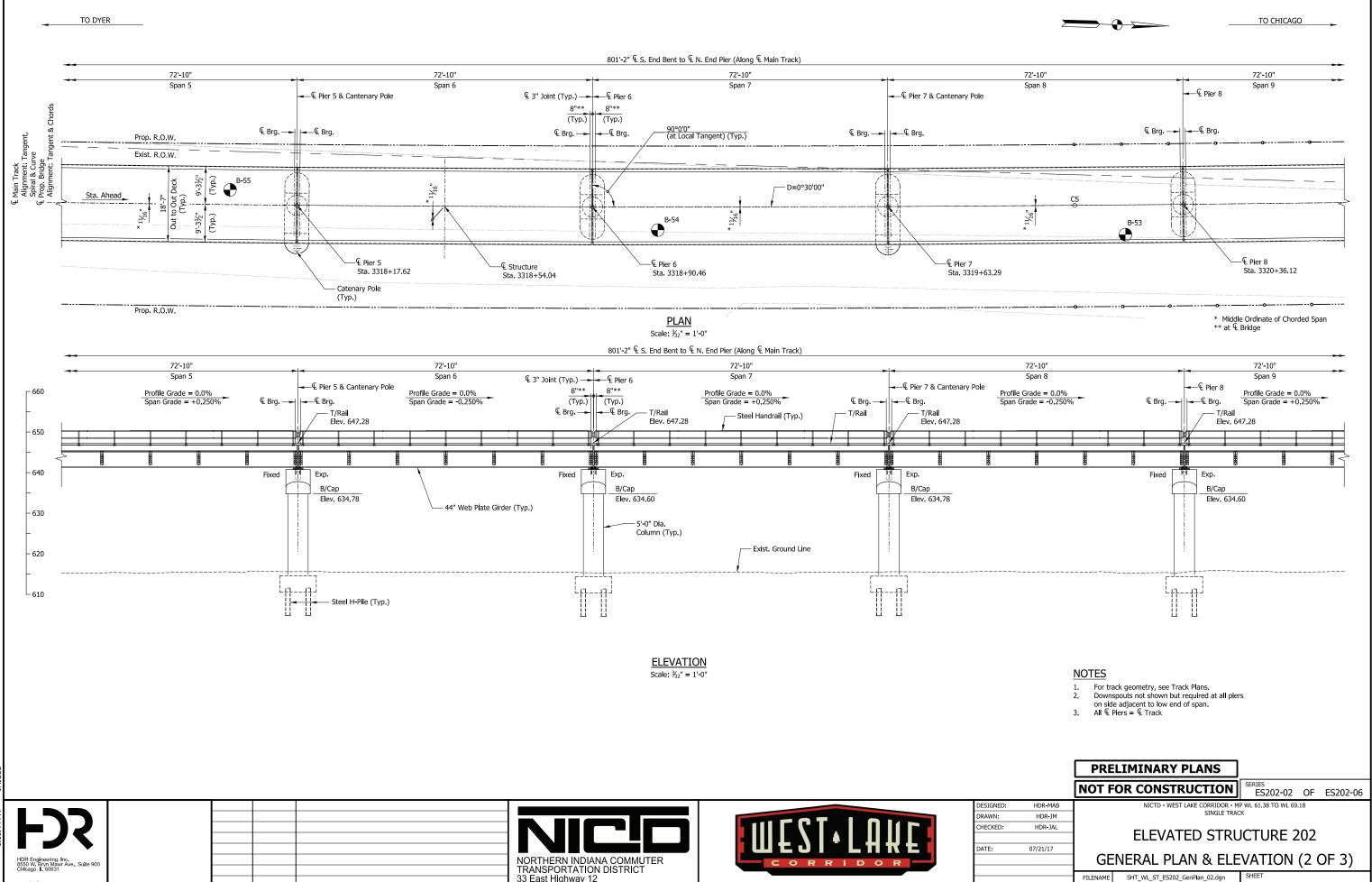
TO CHICAGO







PLOT DATE:



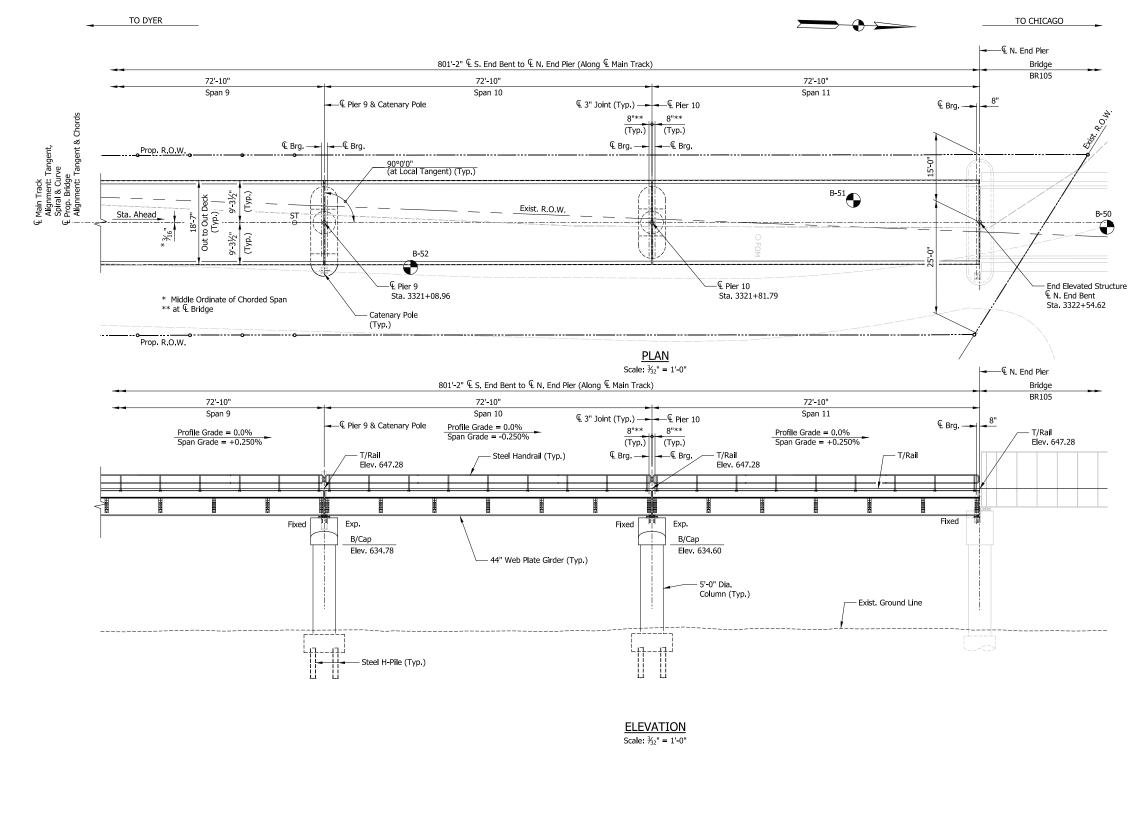
6:0	
	HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 90 Chlcago, IL 60631
5	www.hdrinc.com

ISSUE			NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT 33 East Highway 12 Chesterton, Indiana 46304	DYER TO HAMMOND INDIANA
ISSUE	DATE	DESCRIPTION	Chesterton, Indiana 46304	DYER TO HAMMOND, INDIANA

AS NOTED

SCALE

53 OF 218



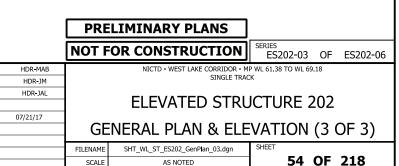
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emt 58							CHECKED:
November 08 6:08:58 PM							
20						μωτοι ιπκε	DATE:
üΰ	HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631				NORTHERN INDIANA COMMUTER	CORRIDOR	
ATE:	Chicago, IL 60631				TRANSPORTATION DISTRICT		
OT D	www.hdrinc.com				33 East Highway 12		
LC LC		ISSUE	DATE	DESCRIPTION	Chesterton, Indiana 46304	DYER TO HAMMOND, INDIANA	

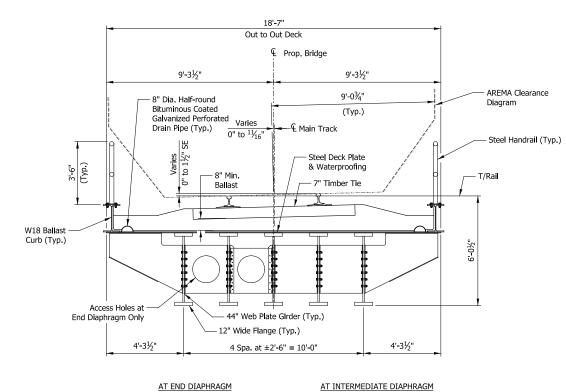
PLOT



<u>NOTES</u>

- 1.
- For track geometry, see Track Plans. Downspouts not shown but required at all piers 2.
- on side adjacent to low end of span.
 All € Piers = € Track

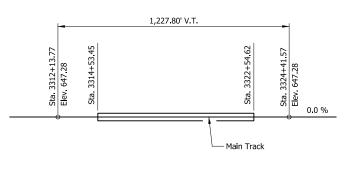




AT END DIAPHRAGM

TYPICAL SECTION

Scale: ¾" = 1'-0"



TOP OF RAIL PROFILE Not to Scale

2 DATE PLOT

HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631

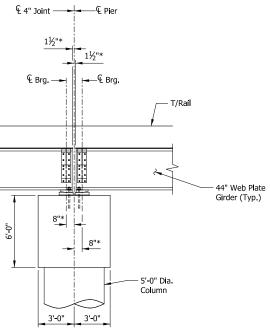
www.hdrinc.com

	· · ·			NORTHERN IND TRANSPORTATI 33 East Highway Chesterton, India
				33 East Highway
	ISSUE	DATE	DESCRIPTION	Chesterton, India
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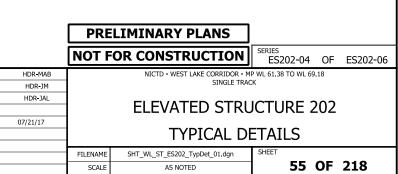


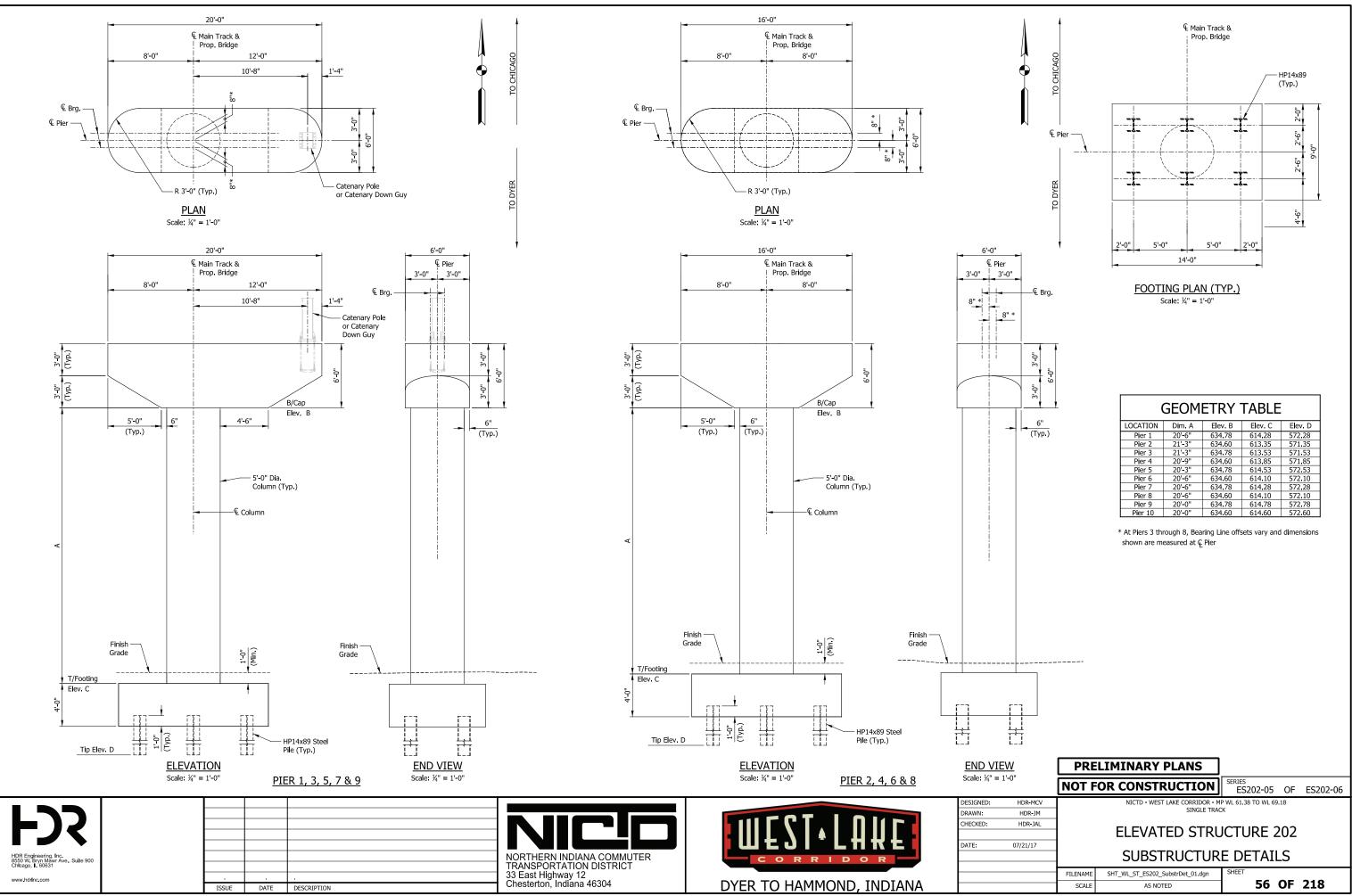


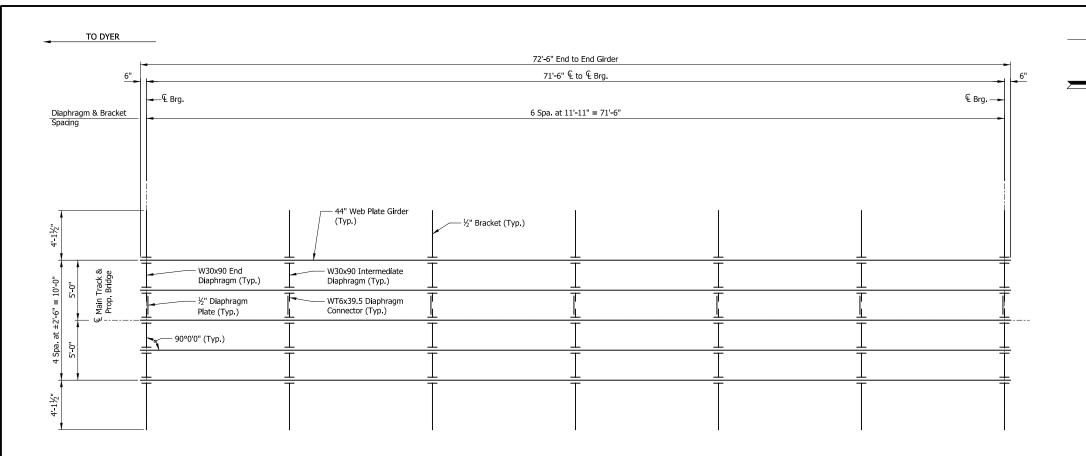
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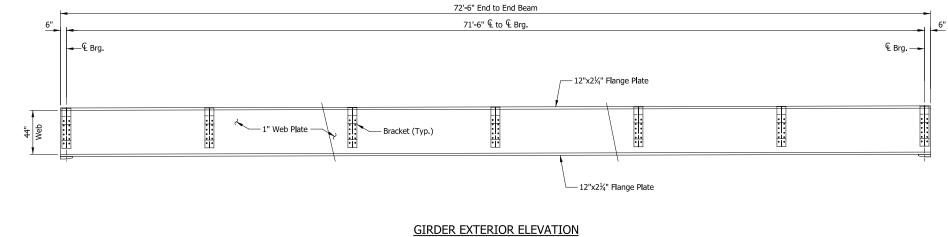
SECTION THRU PIER Scale: ¼" = 1'-0" * at € Bridge







FRAMING PLAN Spans 1 thru 11 Scale: ¼" = 1'-0"



Spans 1 thru 11 Scale: ¹/₄" = 1'-0"

201

80

DATE

РГОТ

			NORTHER
			TRANSPO
			TRANSPO 33 East Hig Chesterton
ISSUE	DATE	DESCRIPTION	Chesterton,

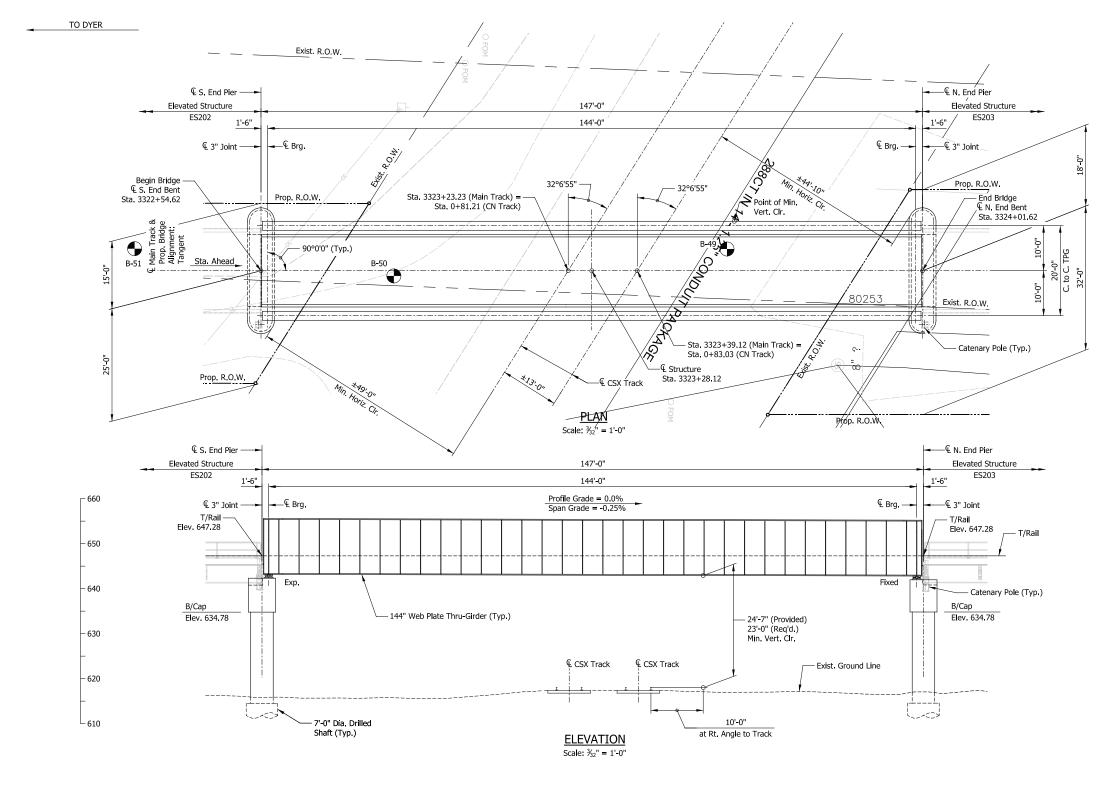




TO CHICAGO



	PRE	LIMINARY PLANS		
	NOT F	OR CONSTRUCTION	SERIES ES202-06 OF ES202-06	
HDR-MAB	NICTD - WEST LAKE CORRIDOR - MP WL 61.38 TO WL 69.18			
HDR-JM	SINGLE TRACK			
HDR-JAL				
		ELEVATED STRU	JUTURE 202	
07/21/17				
	FRAMING PLAN			
	FILENAME	SHT_WL_ST_ES202_FramPlan_01.dgn	SHEET	
	SCALE	AS NOTED	57 OF 218	





PLOT DATE:

TO CHICAGO

DESIGN DATA

SPECIFICATIONS: American Railway Engineering and Maintenance-of-Way Association (A.R.E.M.A.), Manual for Railway Engineering, 2016, Chapter 8 and 15.

LIVE LOADS: Cooper E-50 and Ballast Train per Structural Design Criteria

DEAD LOADS: Maximum Ballast Depth = 19 inches (at High End of Span)

DESIGN STRESSES: Steel: fy = 50,000 psi (ASTM A709 Gr 50) Reinforcing: fy = 60,000 psi (ASTM A615) Concrete: f'c = 4,000 psi (Min.)

SHEET INDEX

BR105-01	General Plan & Elevation
BR105-02	Typical Details
BR105-03	Substructure Details
BR105-04	Framing Plan

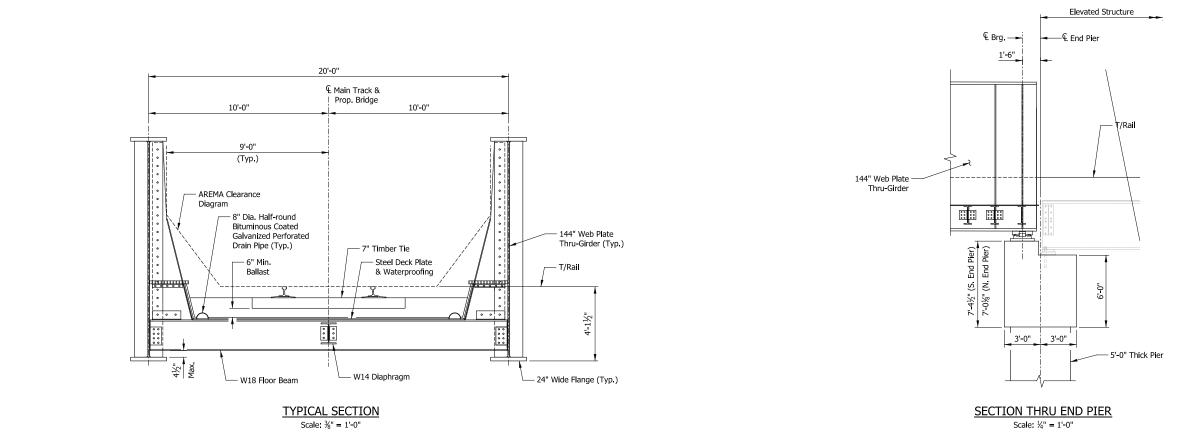
CONSTRUCTION DEPTH

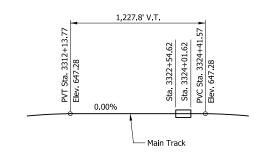
Rail (115 Lbs)	6%"
Tie Plate	34"
Tie	7"
Ballast (min.)	6"
Waterproofing	1"
Deck Plate	34"
Floorbeam	20%"
Bracing	4½"
Bottom Flange	2½"

49½"

NOTES

- For track geometry, see Track Plans.
 Downspouts not shown but required at all piers
- Downspouts not shown but required at all piers on side adjacent to low end of span.





TOP OF RAIL PROFILE Not to Scale

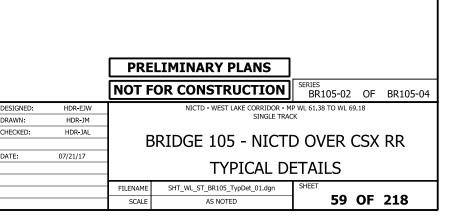
2 DATE РГОТ

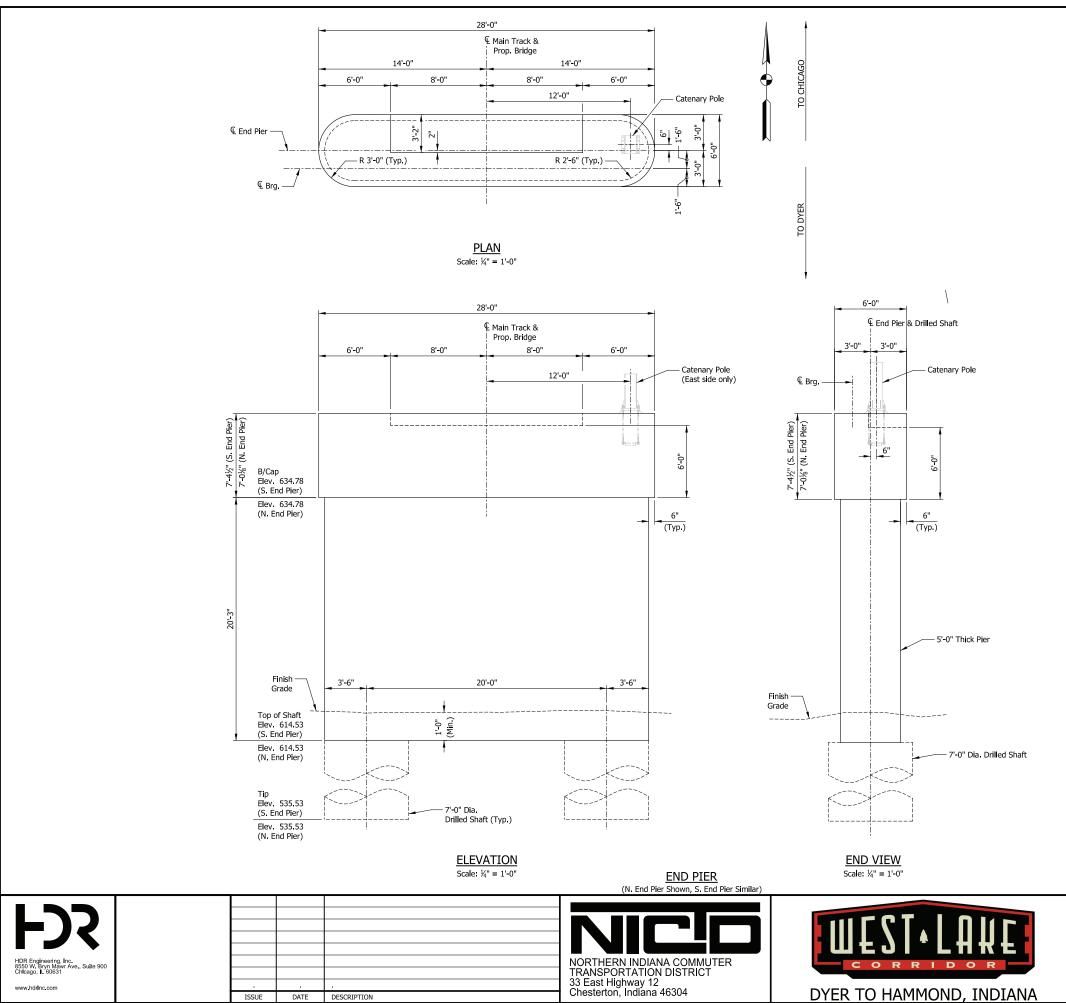
HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631

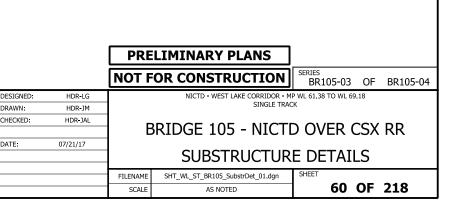
www.hdrinc.com

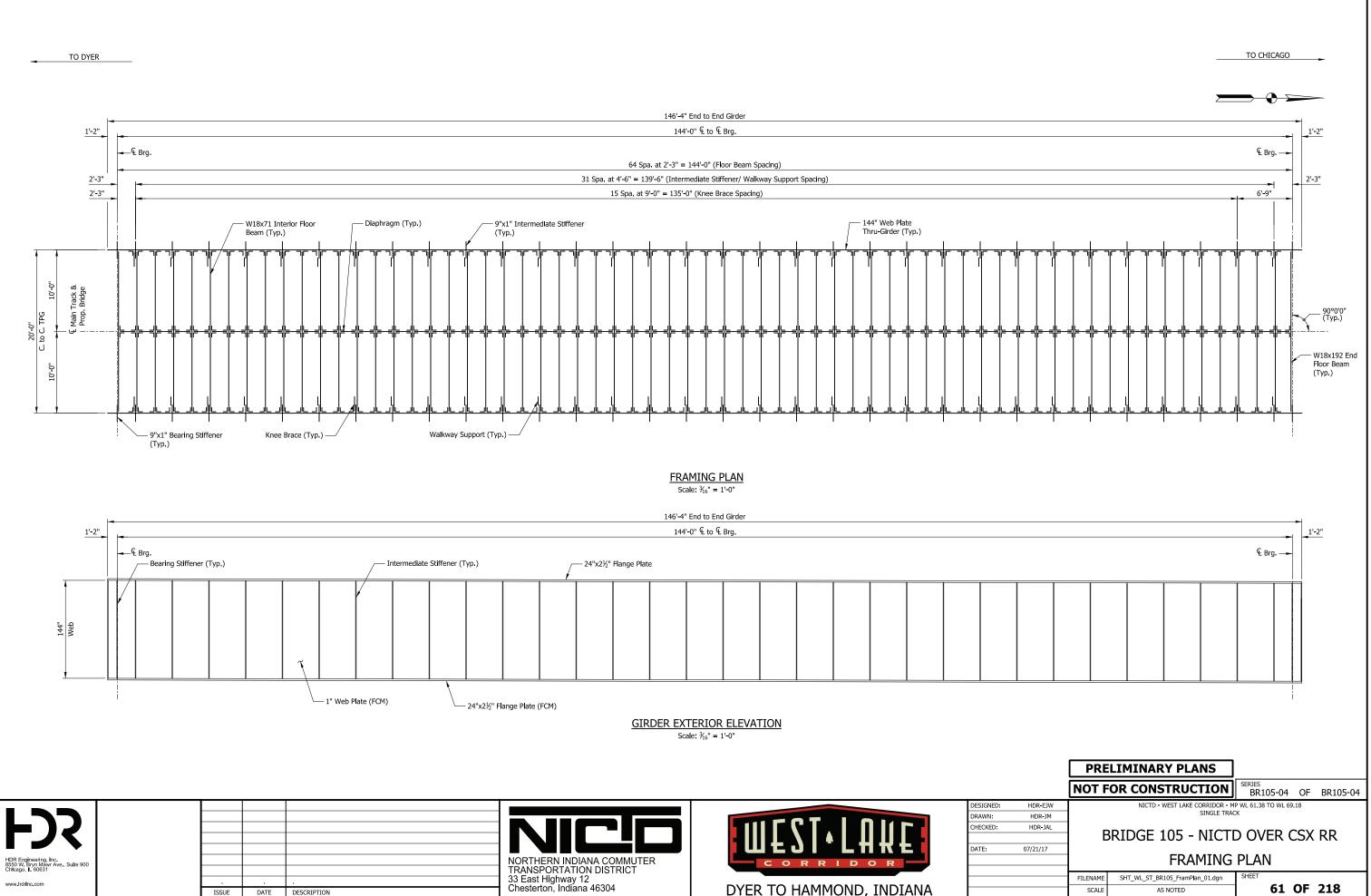
	DATE		NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT 33 East Highway 12 Chesterton, Indiana 46304	
ISSUE	DATE	DESCRIPTION		



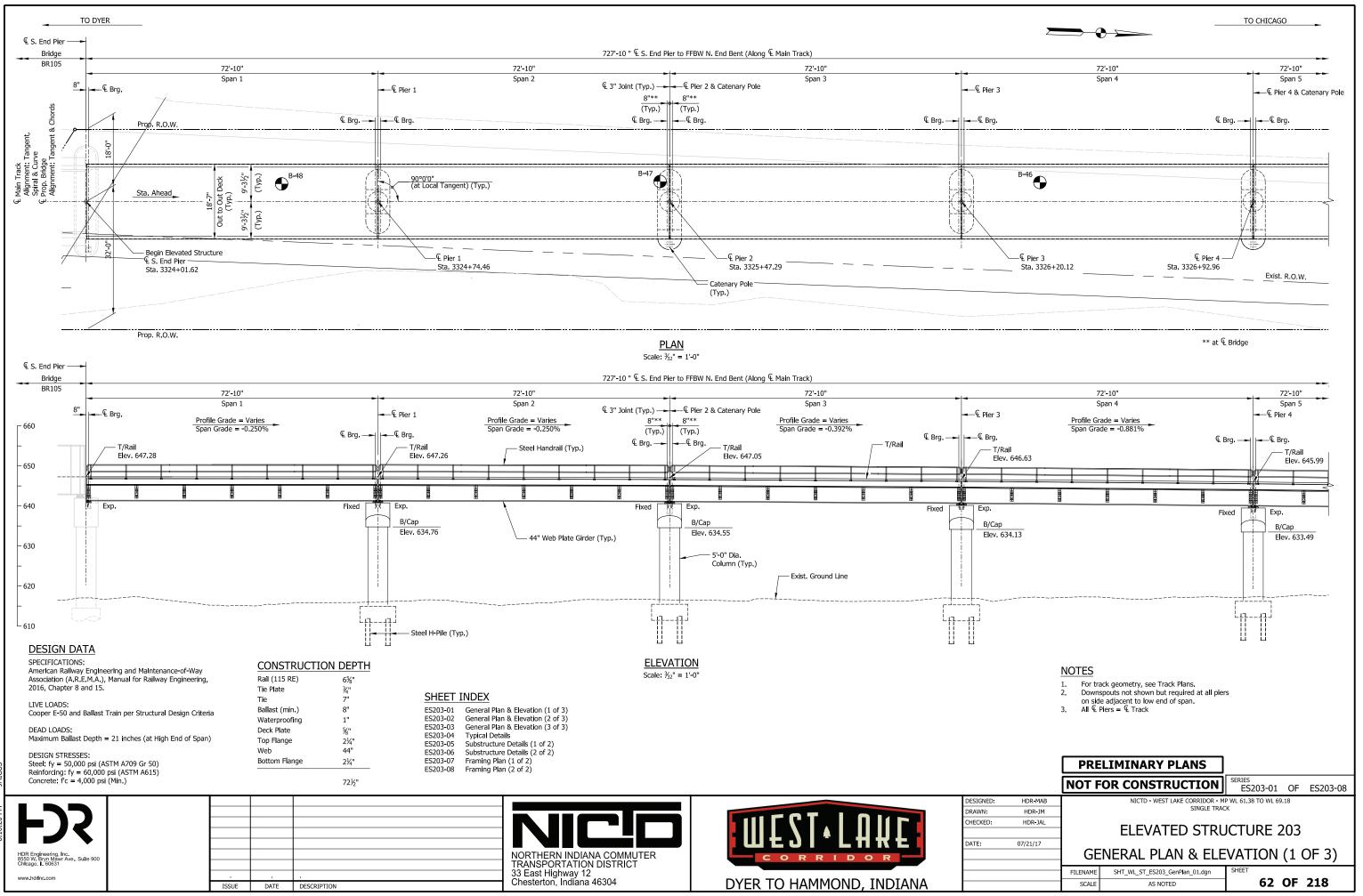




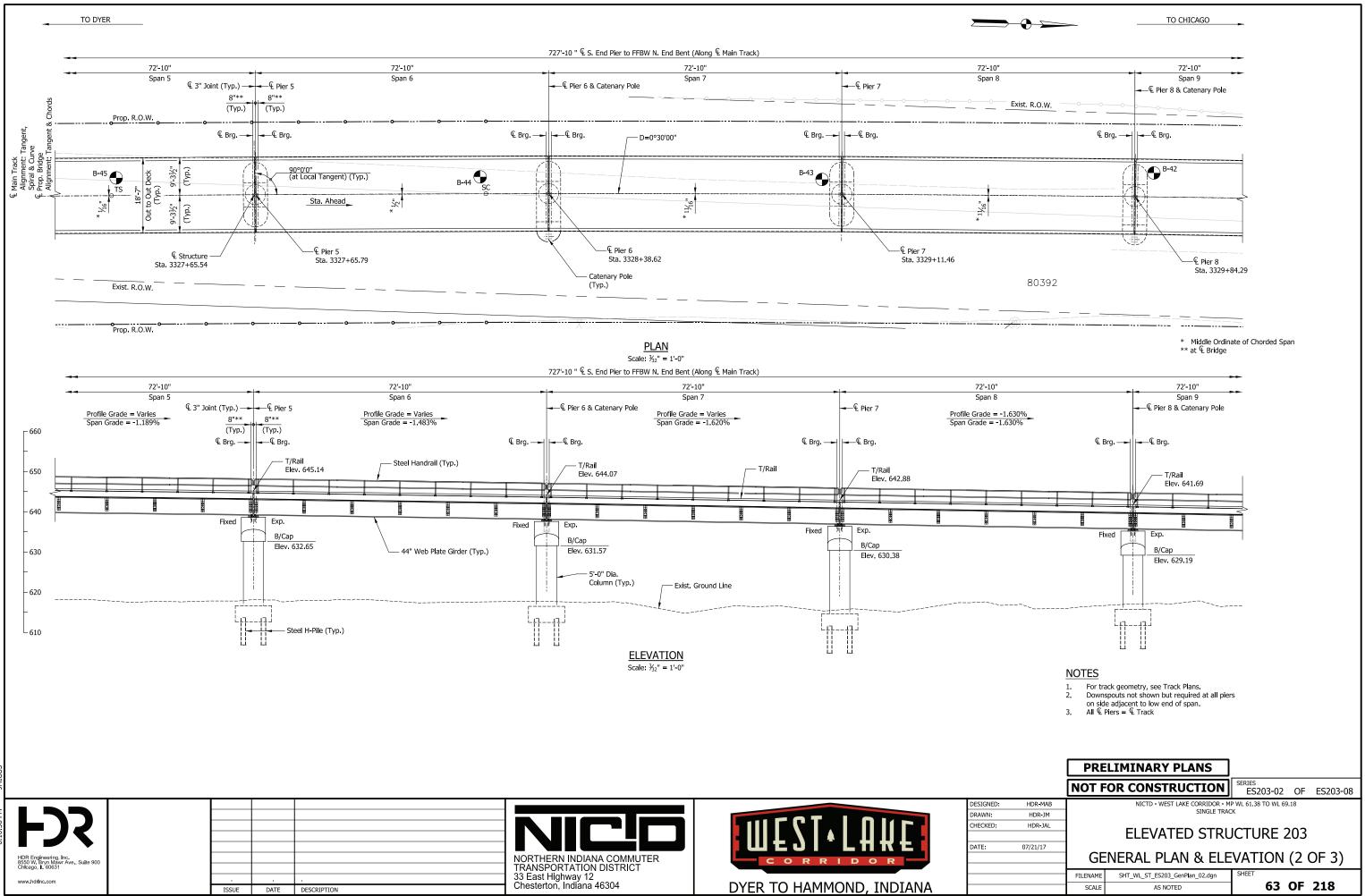


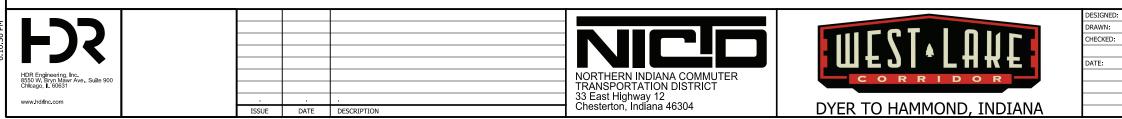


DYER TO HAMMOND, INDIANA



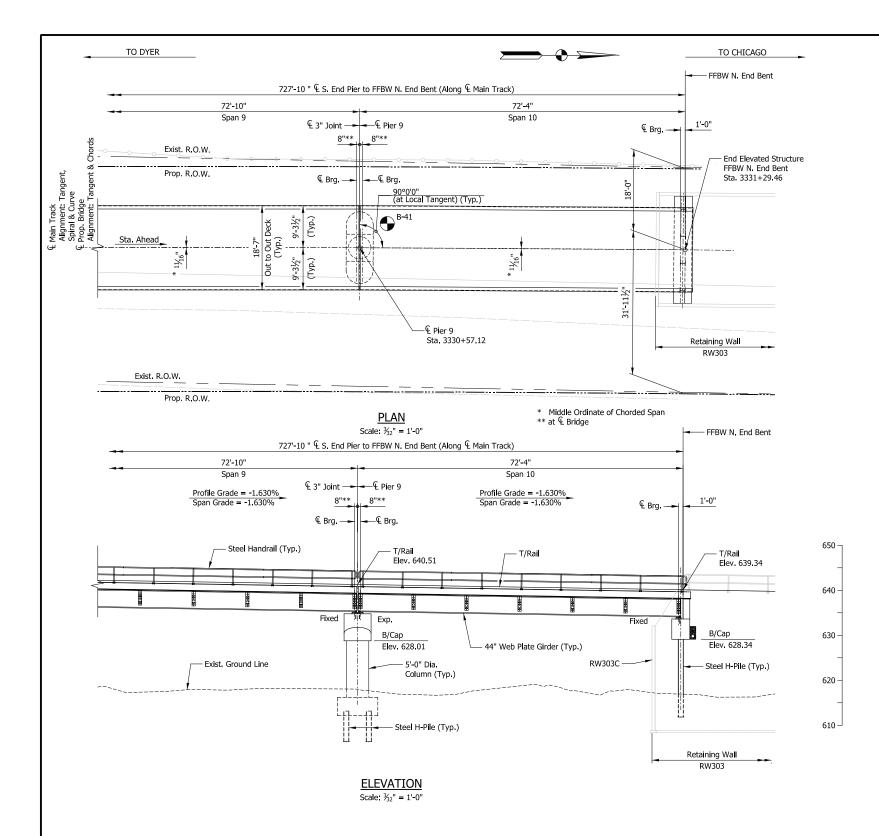
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HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631 www.hdrinc.com

DATE

PLOT

			NORTHERN
			TRANSPOR
			TRANSPOR 33 East High Chesterton,
ISSUE	DATE	DESCRIPTION	Chesterton,

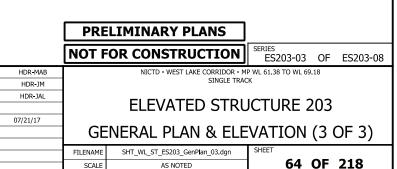


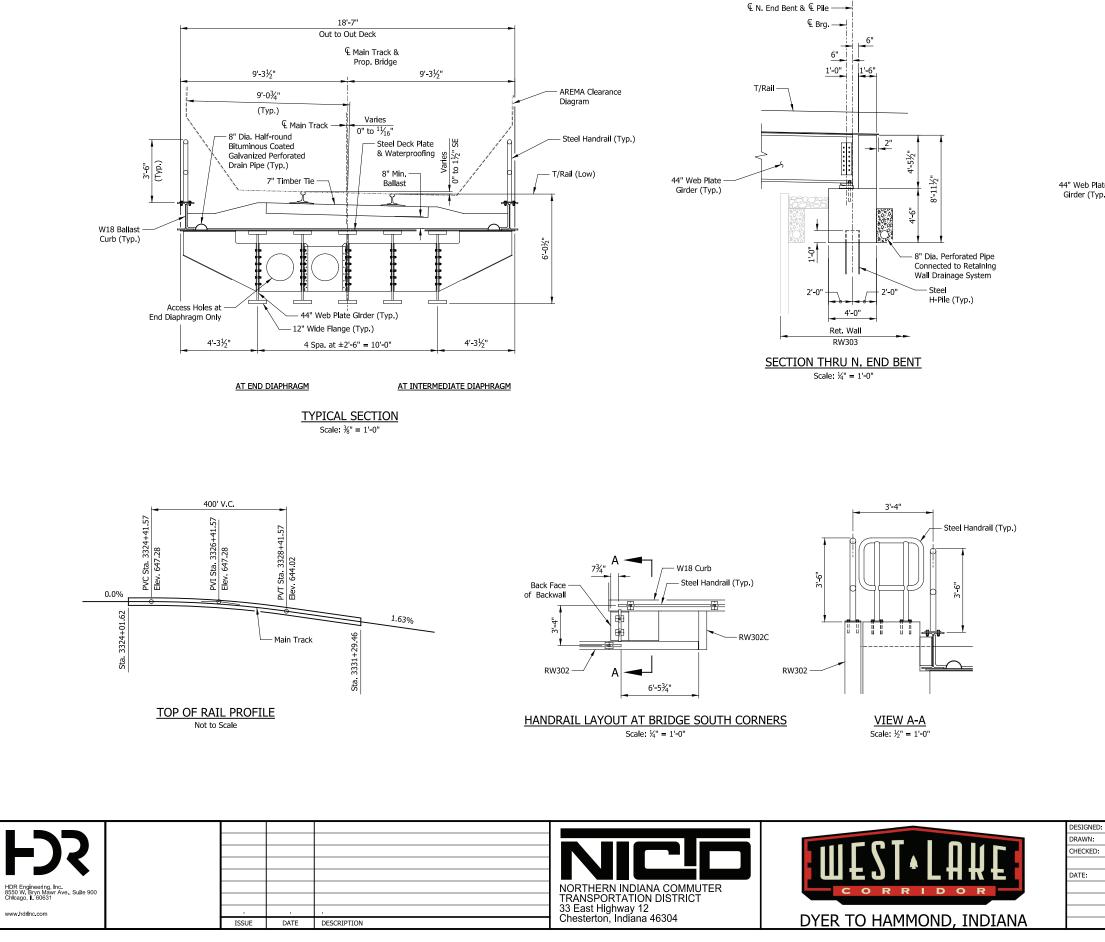


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<u>NOTES</u>

- 1.
- For track geometry, see Track Plans. Downspouts not shown but required at all piers 2.
- on side adjacent to low end of span.
 All € Piers = € Track

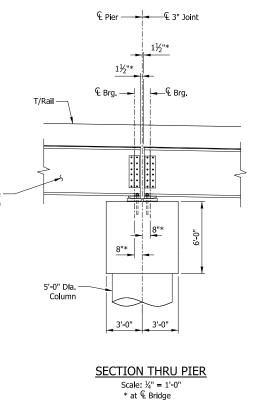




44" Web Plate Girder (Typ.)

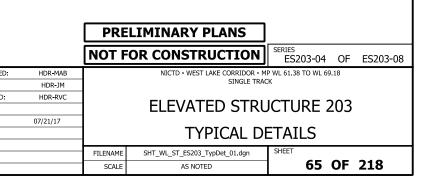
DATE PLOT

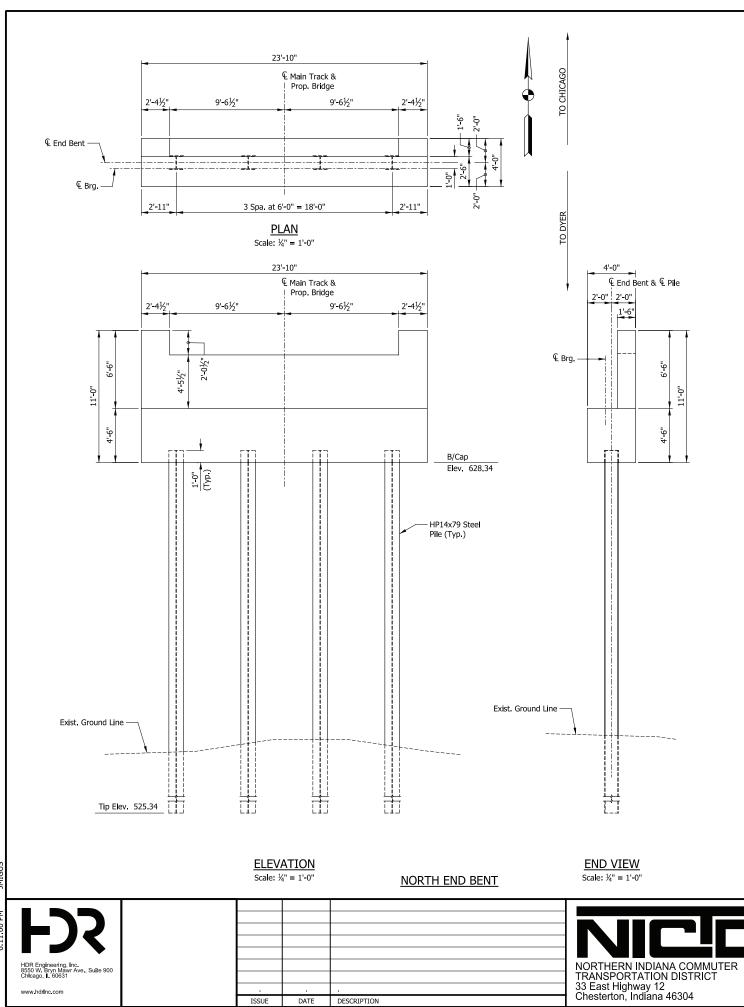
www.hdrinc.com





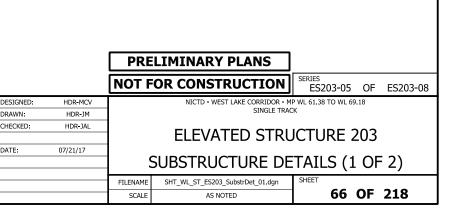
1. Outfall from Drainage System to be routed and detailed similar to those provided for Retaining Wall Underdrains.

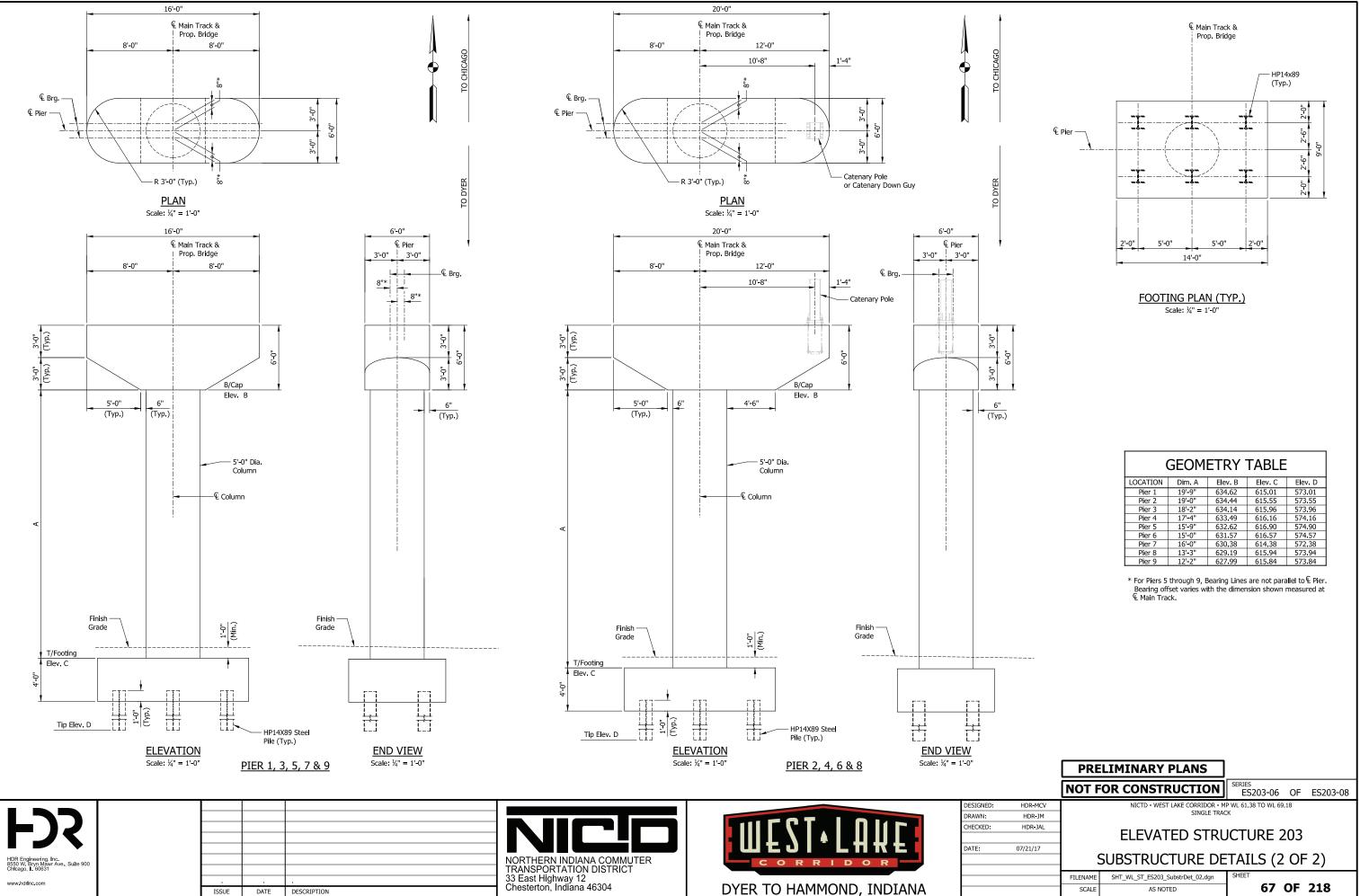


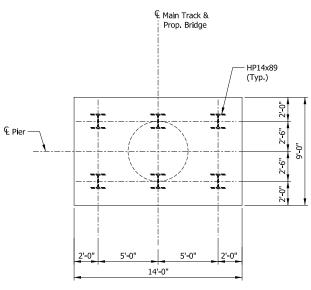




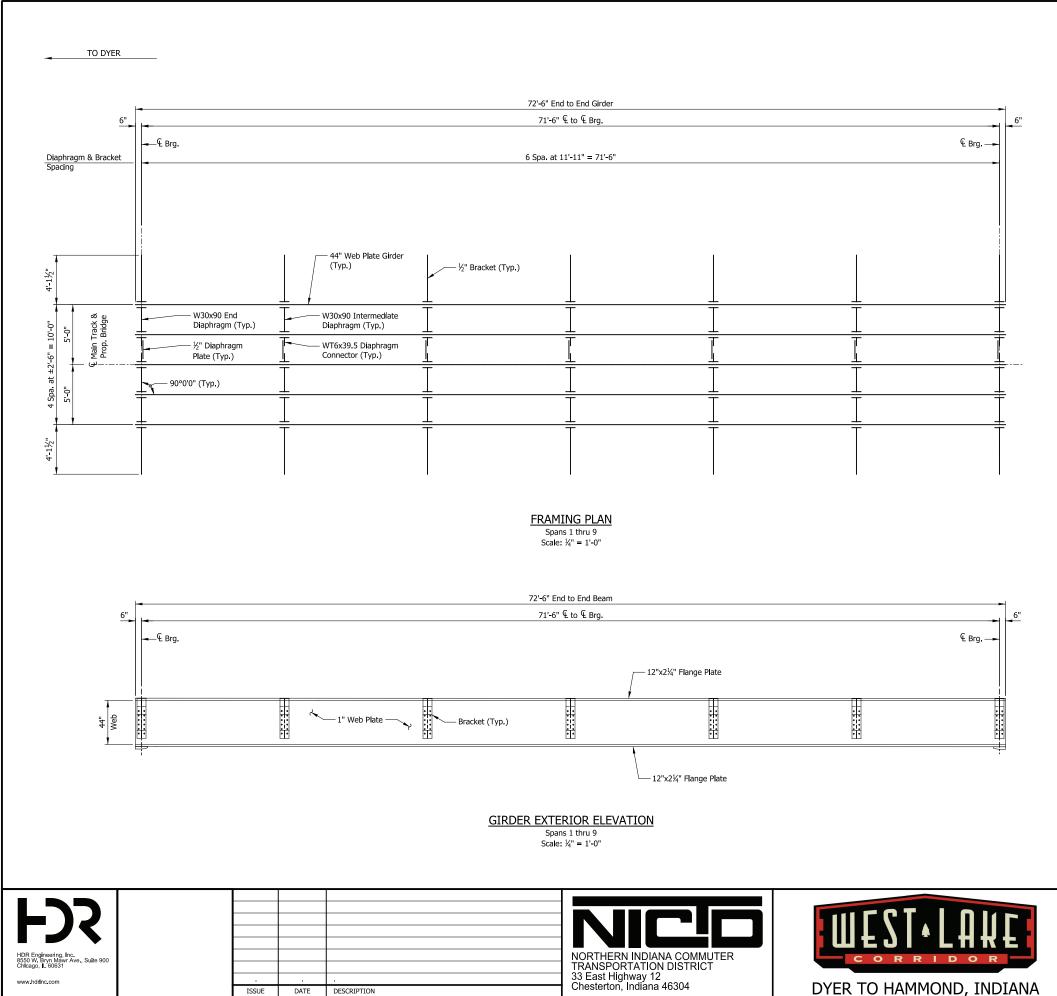
PLOT DATE: November 08, 2017







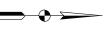
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LOCATION	Dim. A	Elev. B	Elev. C	Elev. D	
Pier 1	19'-9"	634.62	615.01	573.01	
Pier 2	19'-0"	634.44	615.55	573.55	
Pier 3	18'-2"	634.14	615.96	573.96	
Pier 4	17'-4"	633.49	616.16	574.16	
Pier 5	15'-9"	632.62	616.90	574.90	
Pier 6	15'-0"	631.57	616.57	574.57	
Pier 7	16'-0"	630.38	614.38	572.38	
Pier 8	13'-3"	629.19	615.94	573.94	
Pier 9	12'-2"	627.99	615.84	573.84	

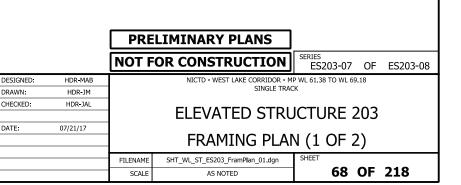


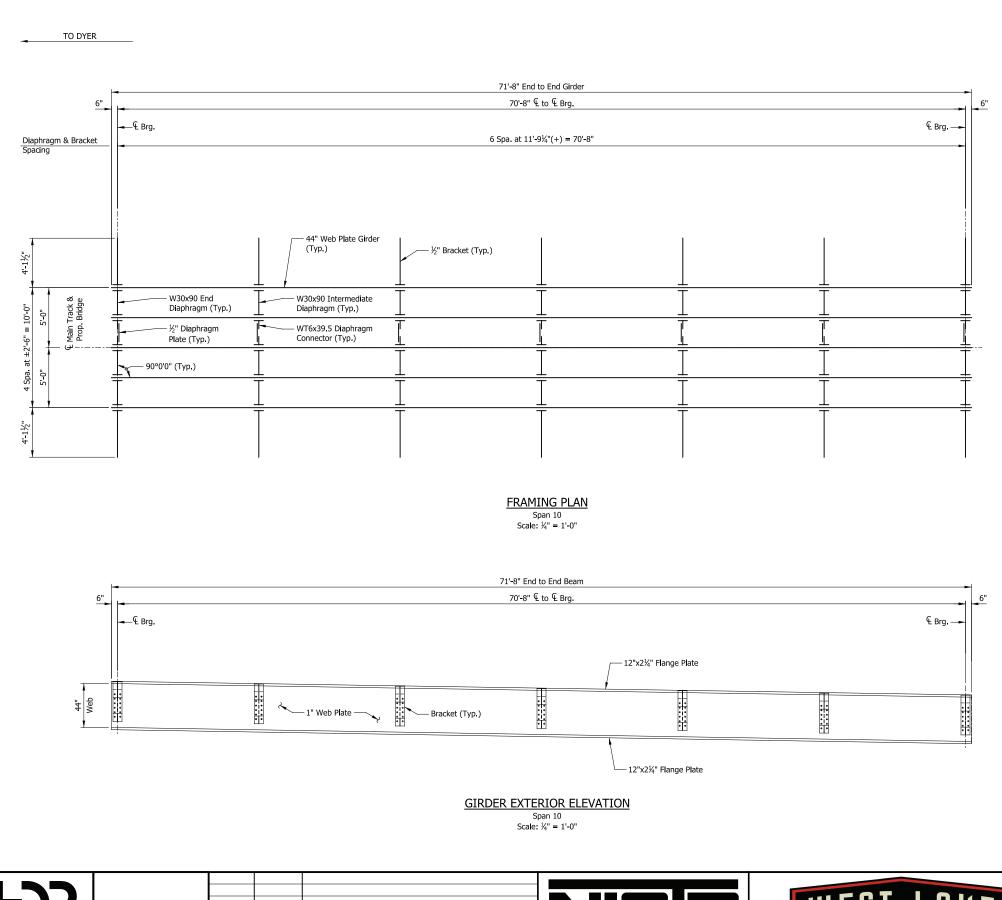
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DATE DESCRIPTION

TO CHICAGO







PLOT

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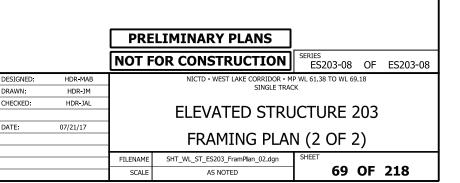
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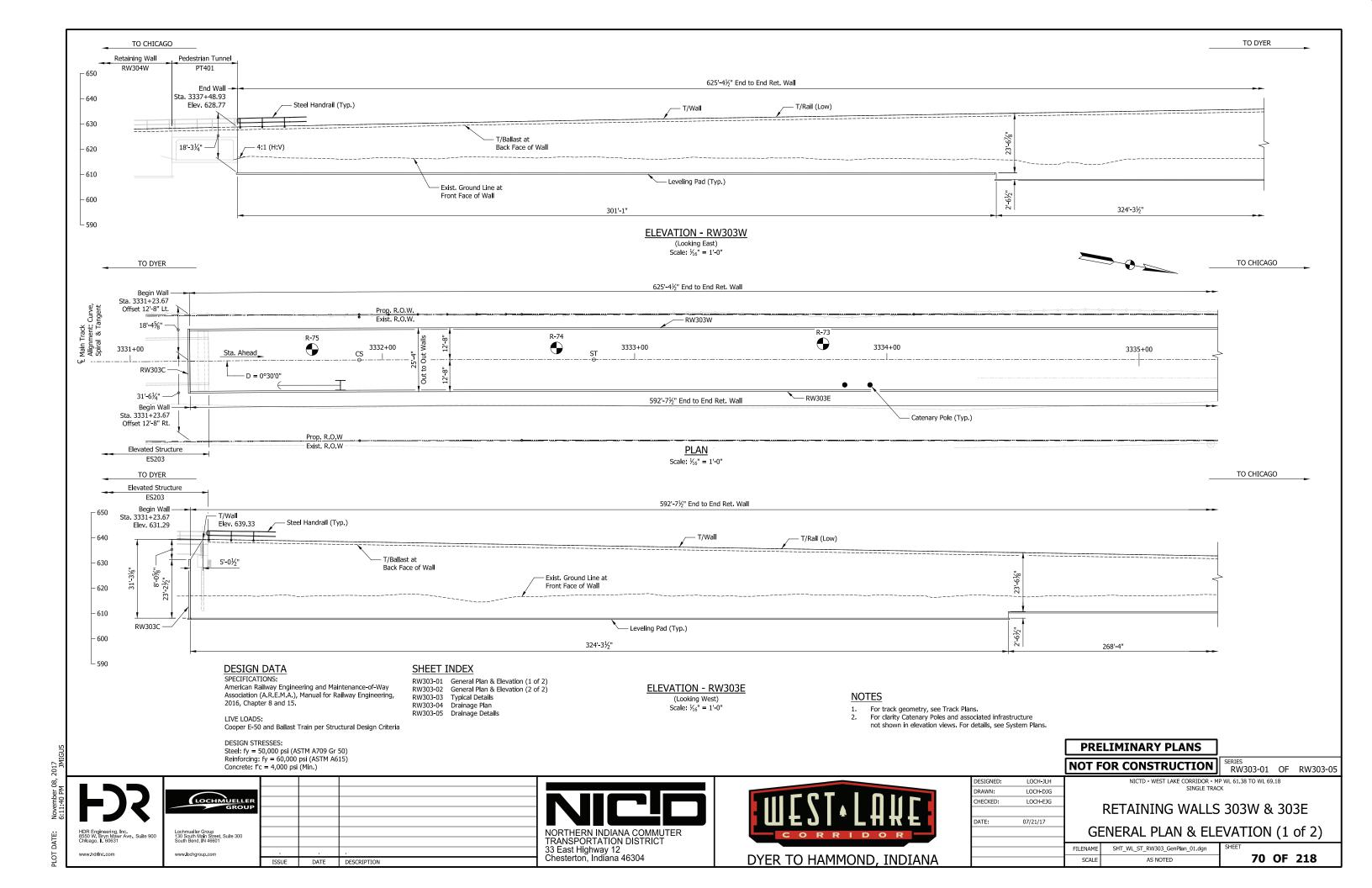
NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT 33 East Highway 12 Chesterton, Indiana 46304

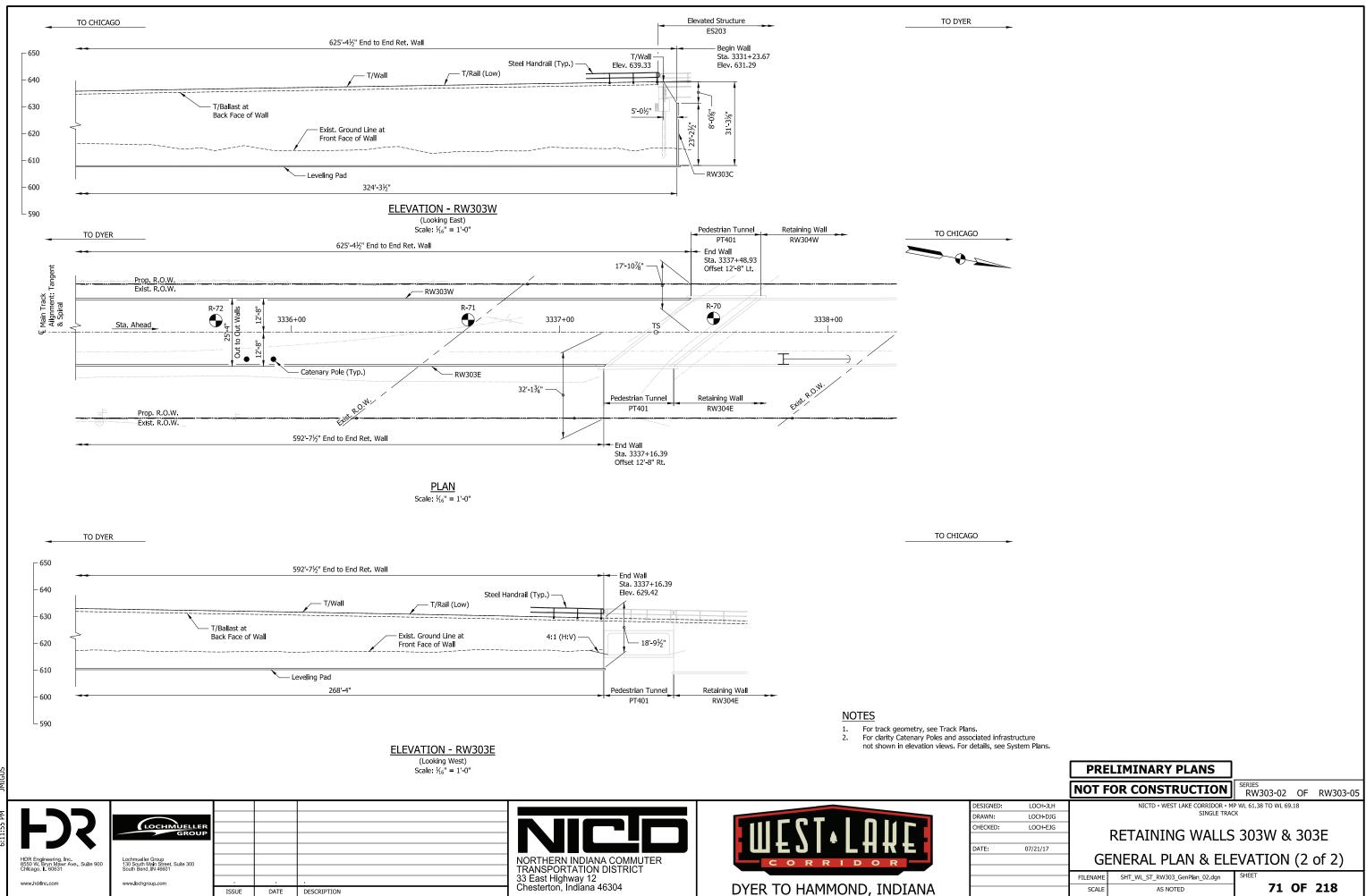


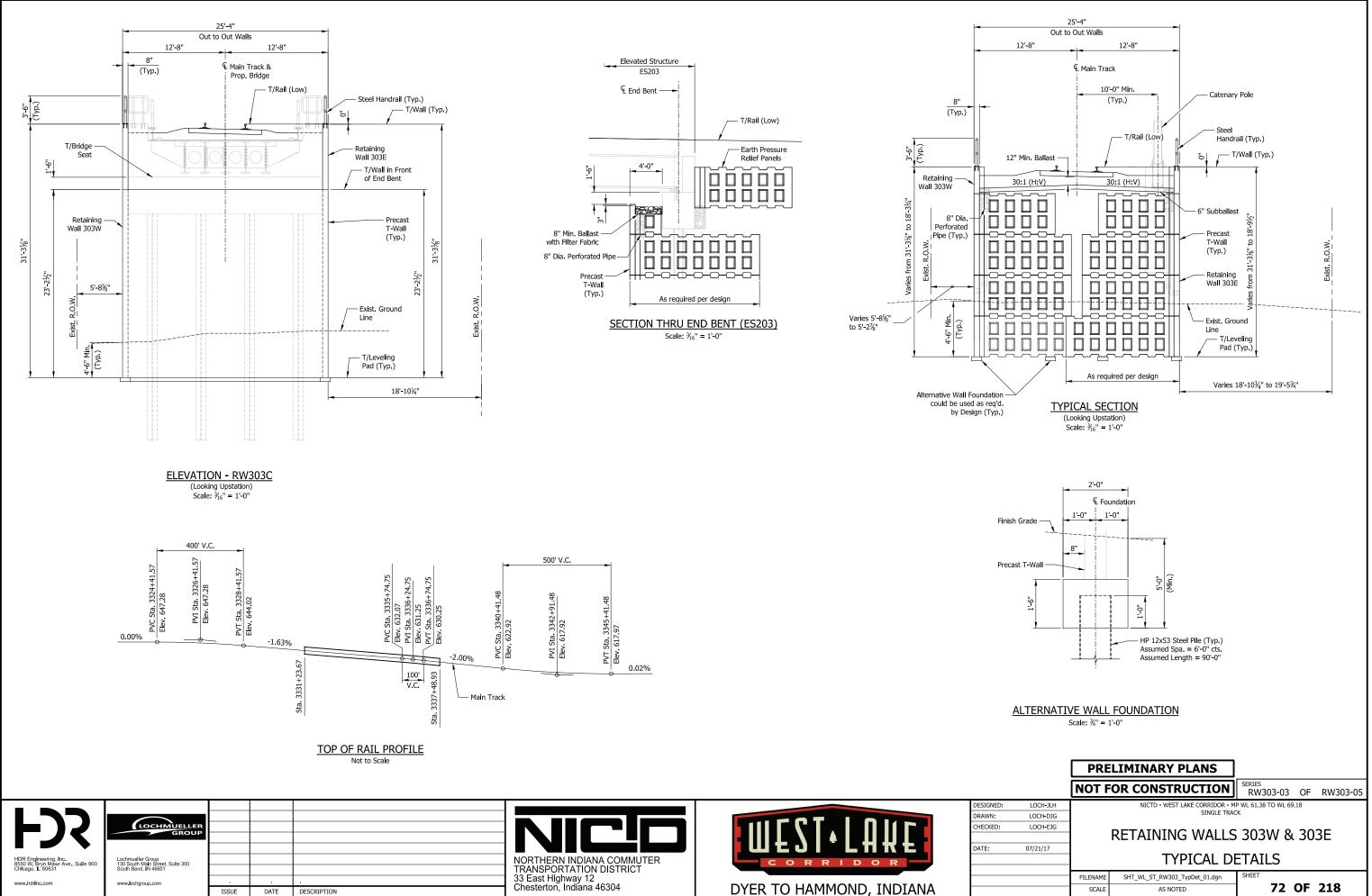
TO CHICAGO



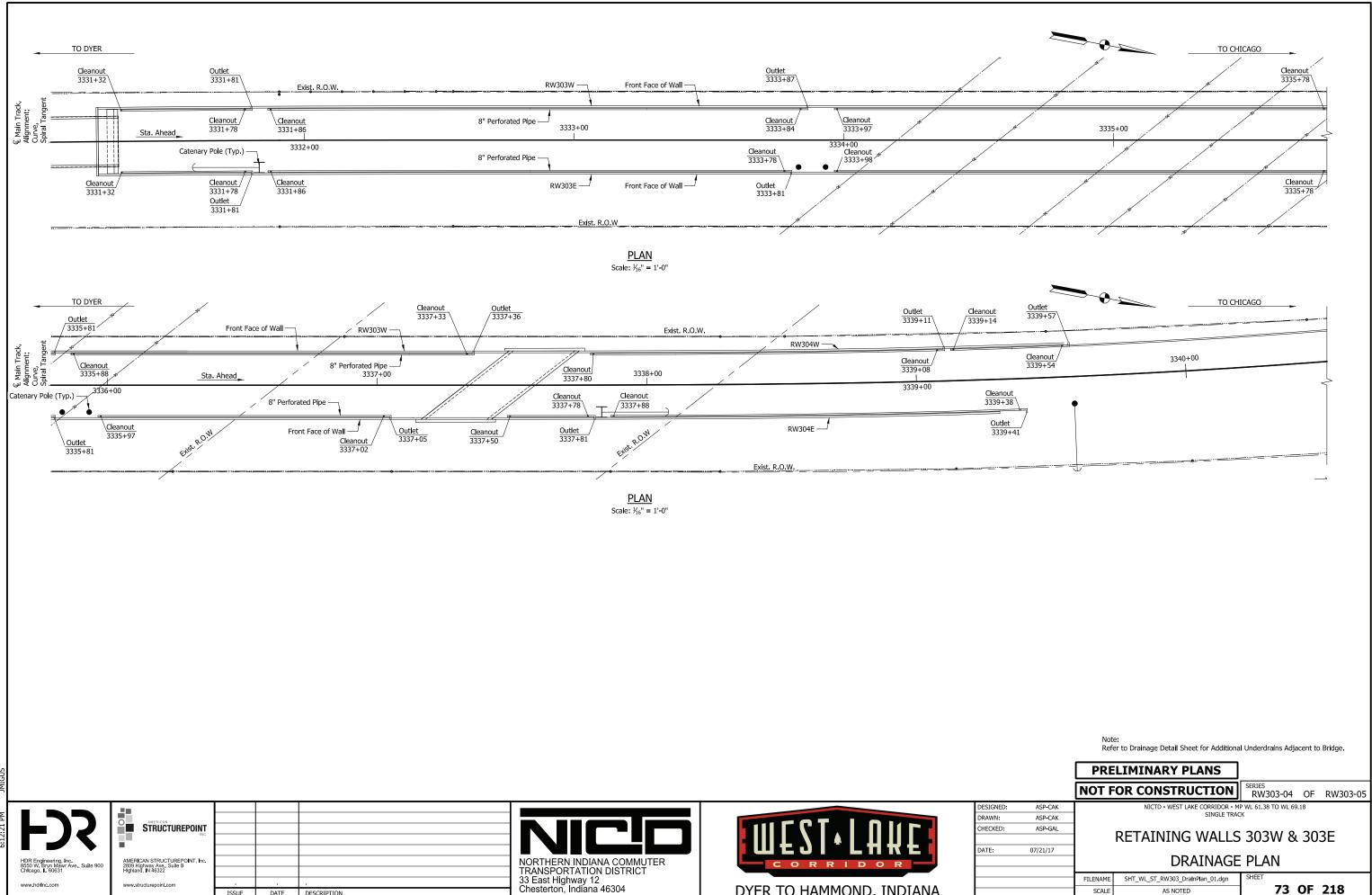








	NORTHERN INDIANA TRANSPORTATION I 33 East Highway 12 Chesterton, Indiana 4
IPTION	Chesterton, Indiana 4

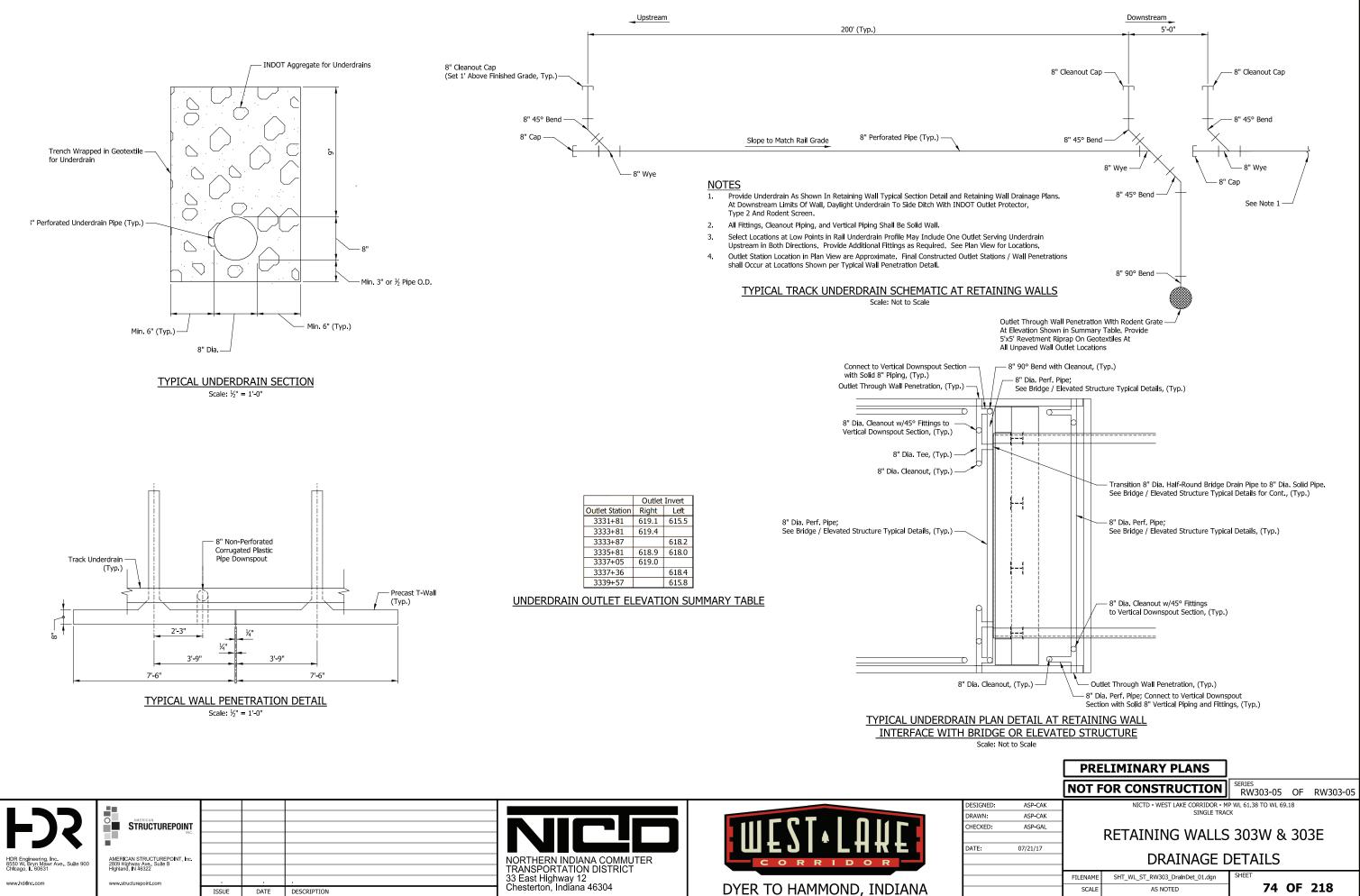


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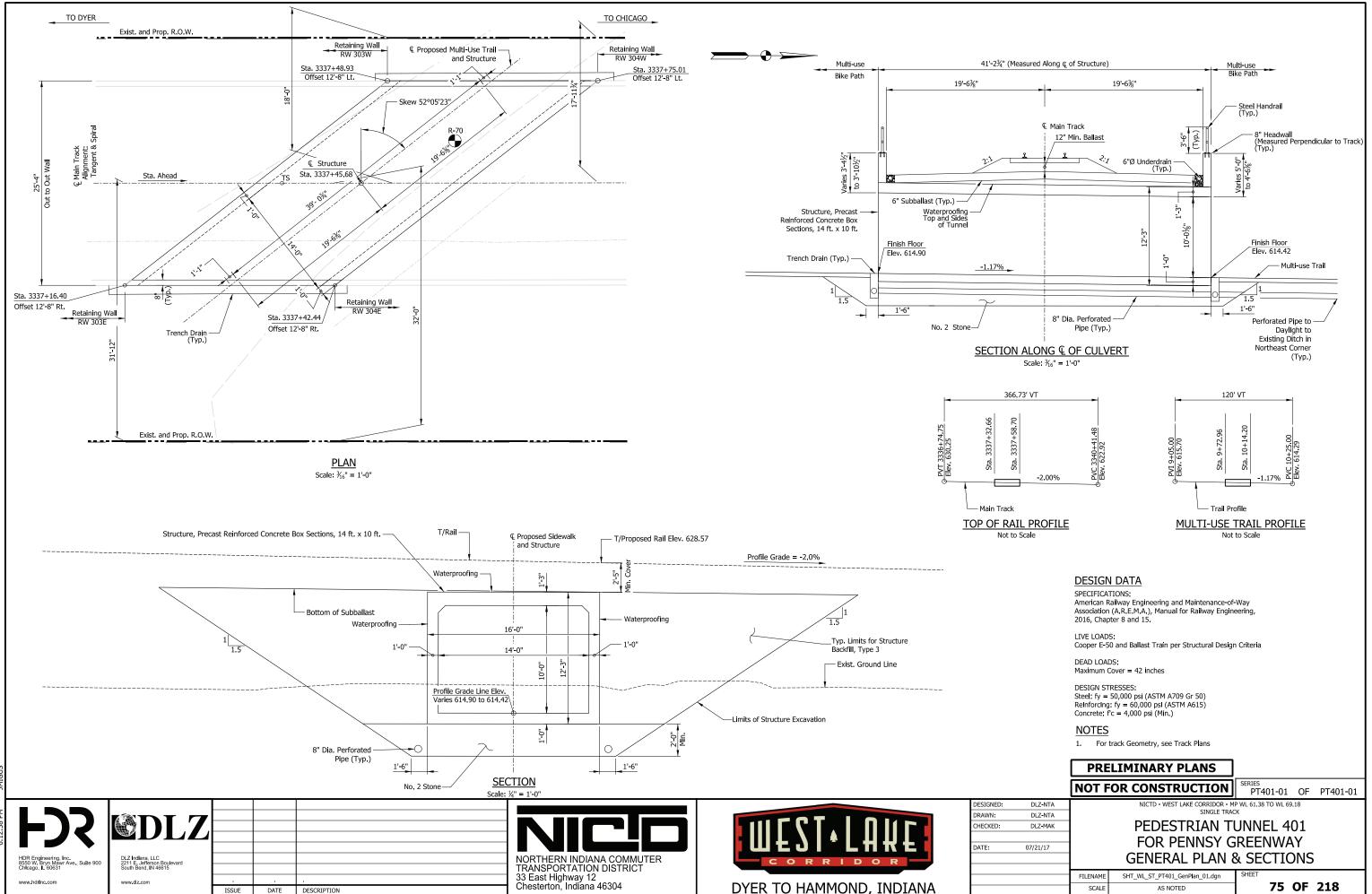
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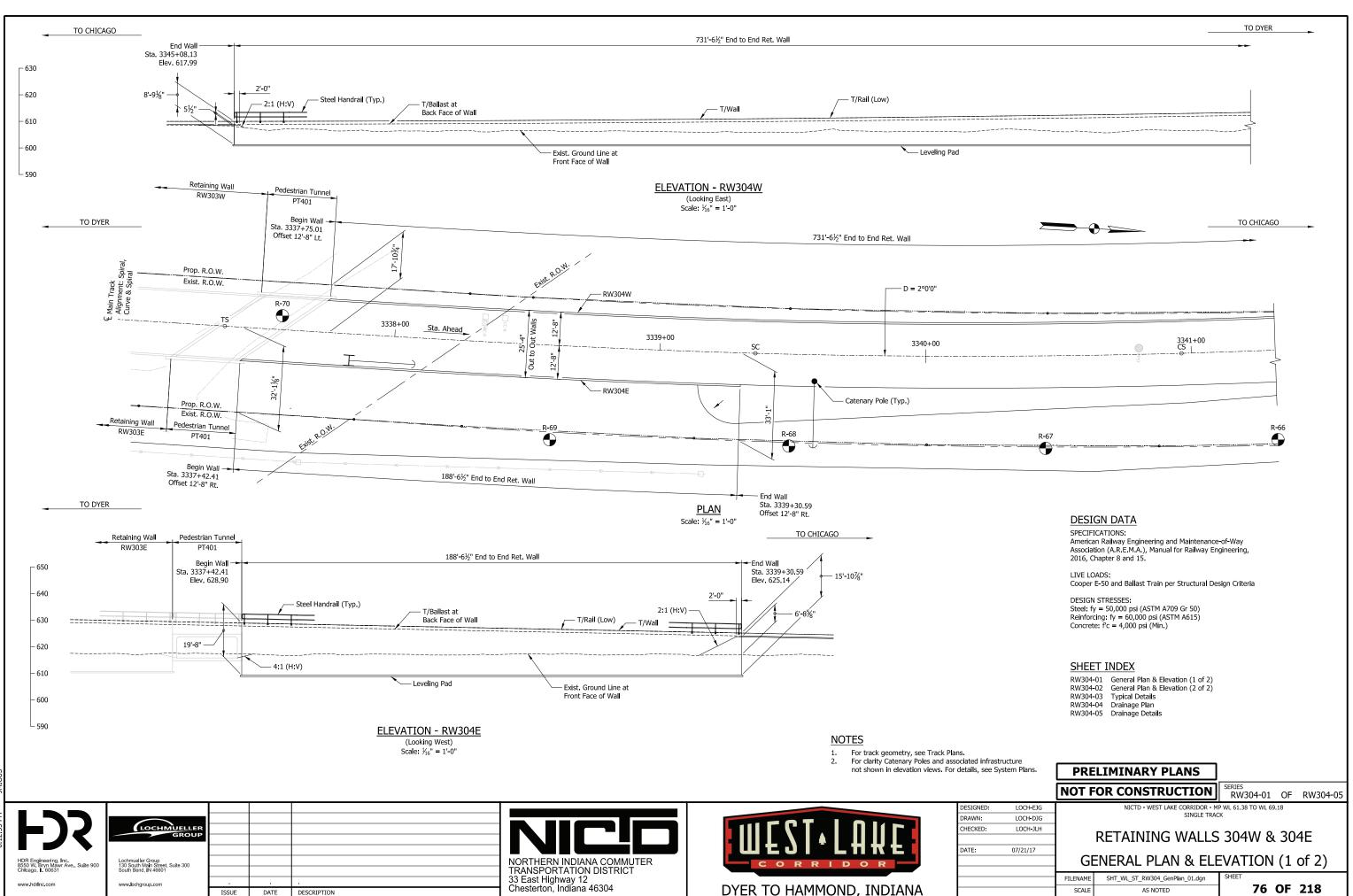
DYER TO HAMMOND, INDIANA

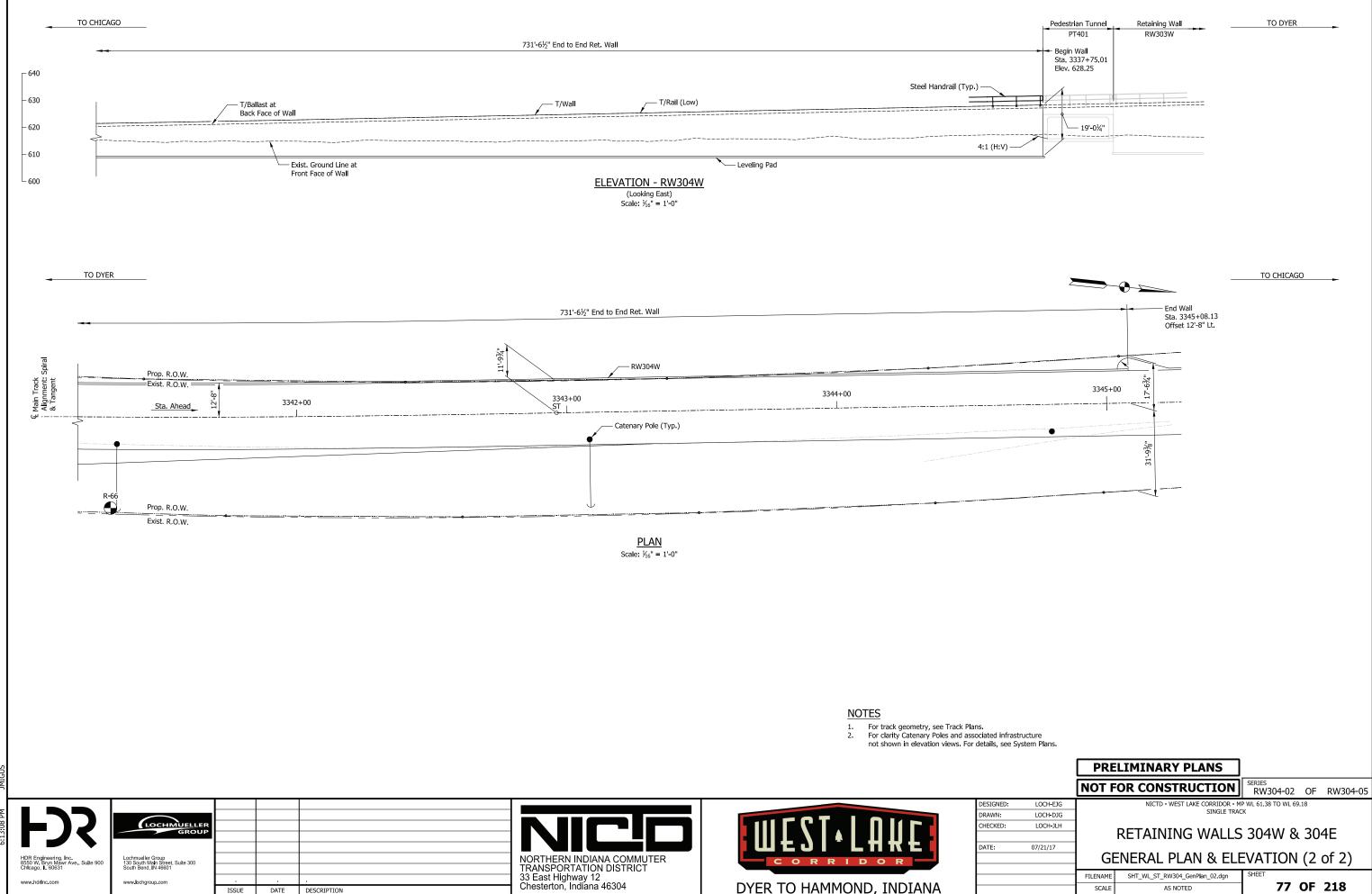


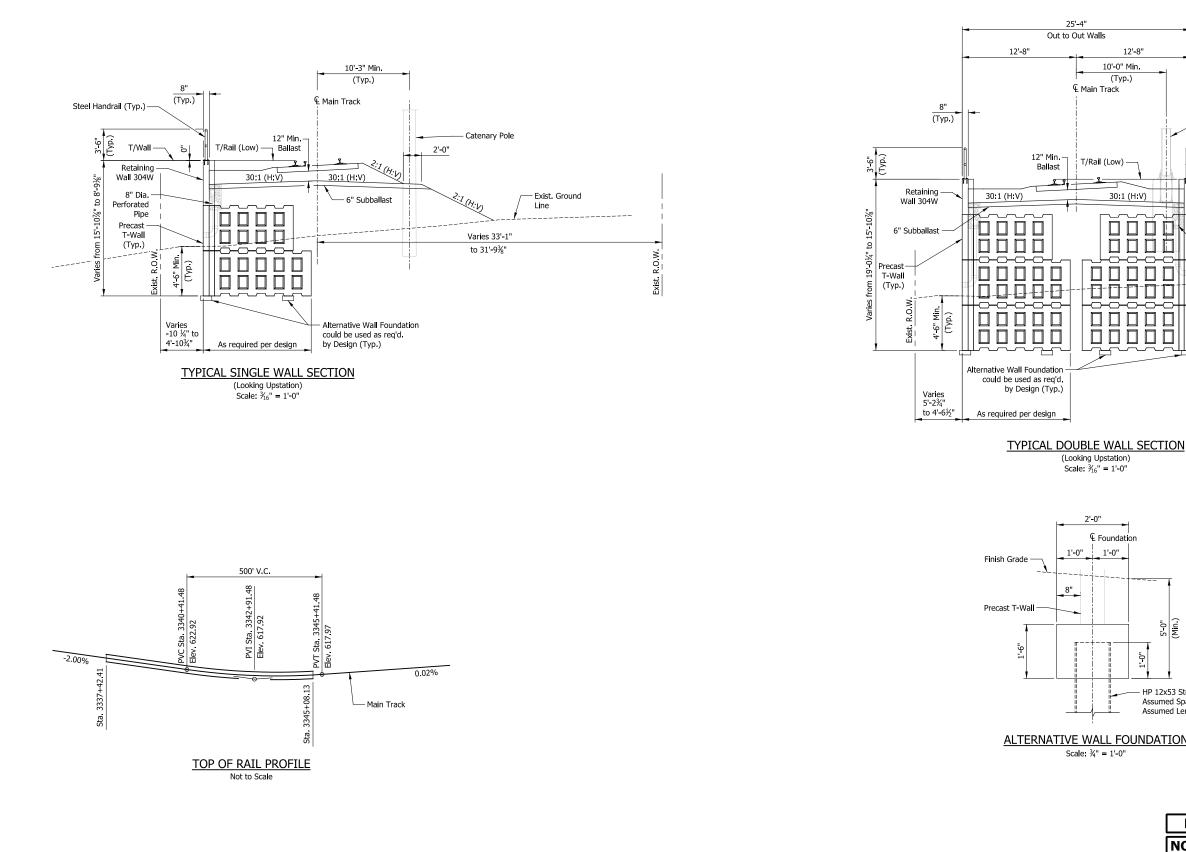




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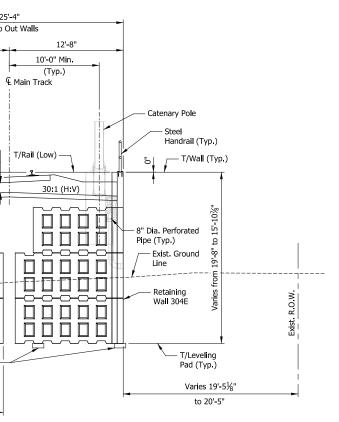


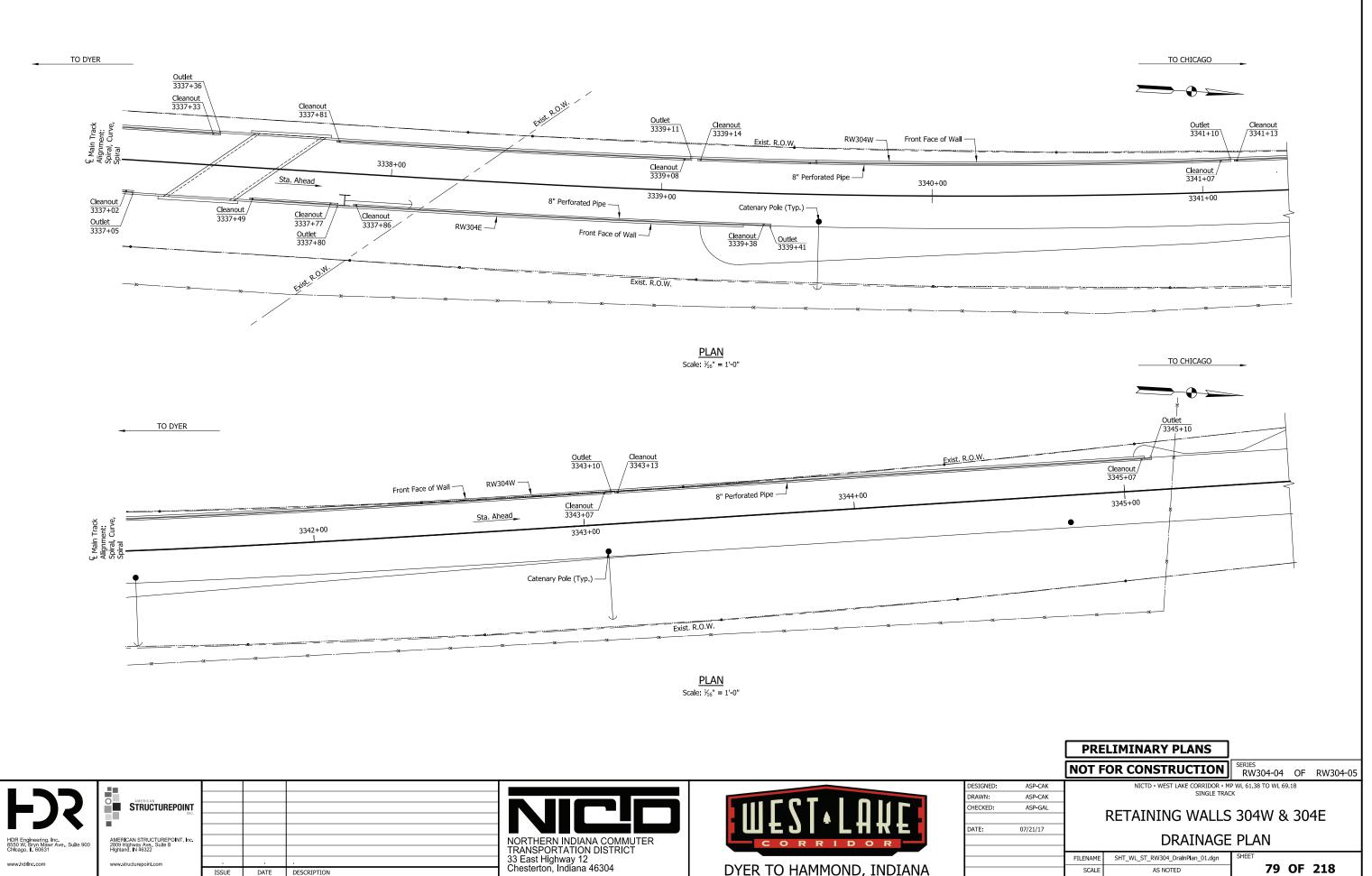
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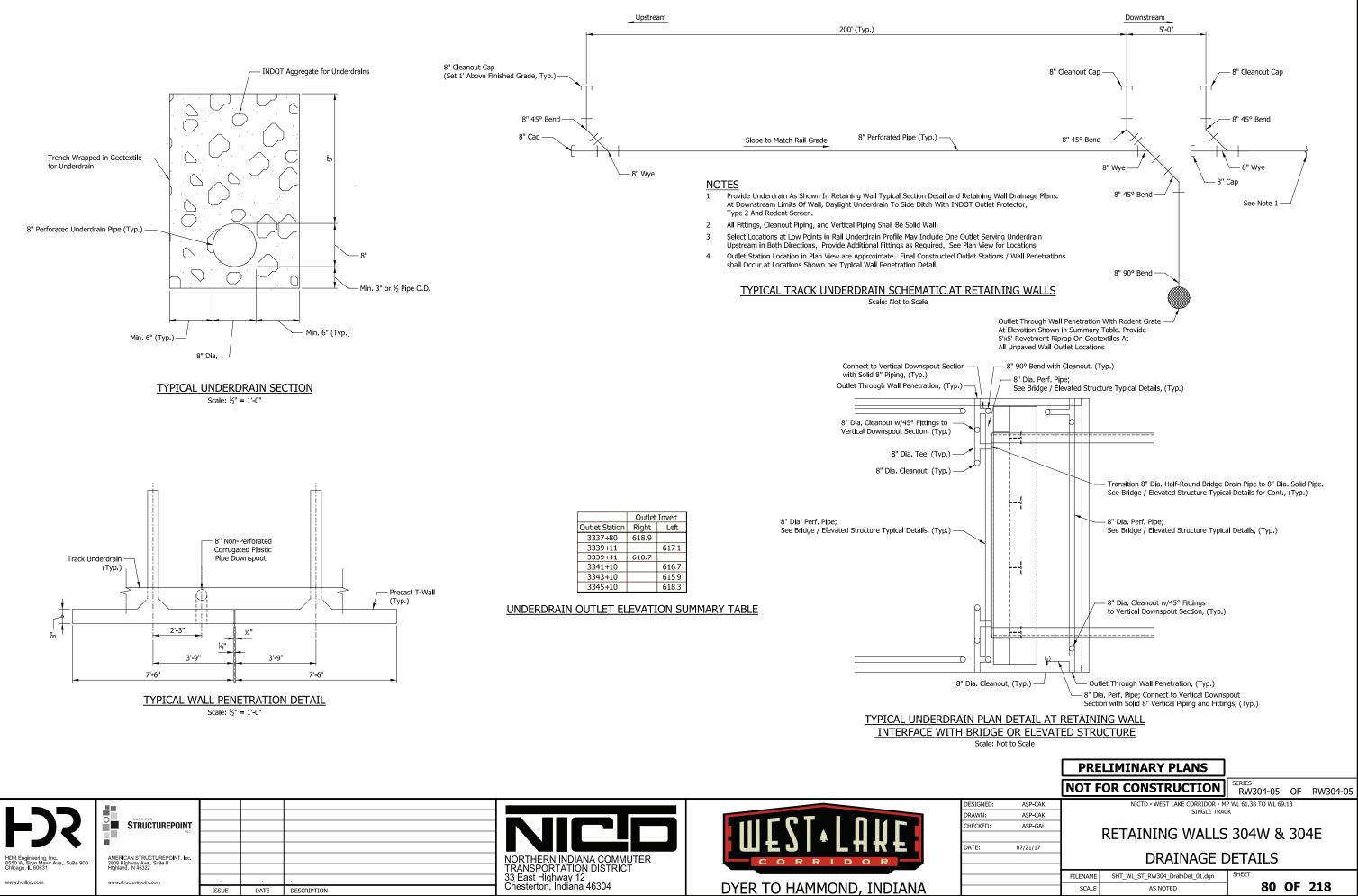
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ISSUE DATE DESCRIPTION Chesterton, Indiana 46304 DYER TO HA	GROUP Lochmueller Group 130 South Main Street, Suite 300 South Bend, IN 46601				TRANSPORTATION DISTRICT	
	www.aongroup.com	ISSUE	DATE	DESCRIPTION	Chesterton, Indiana 46304	<u> </u>

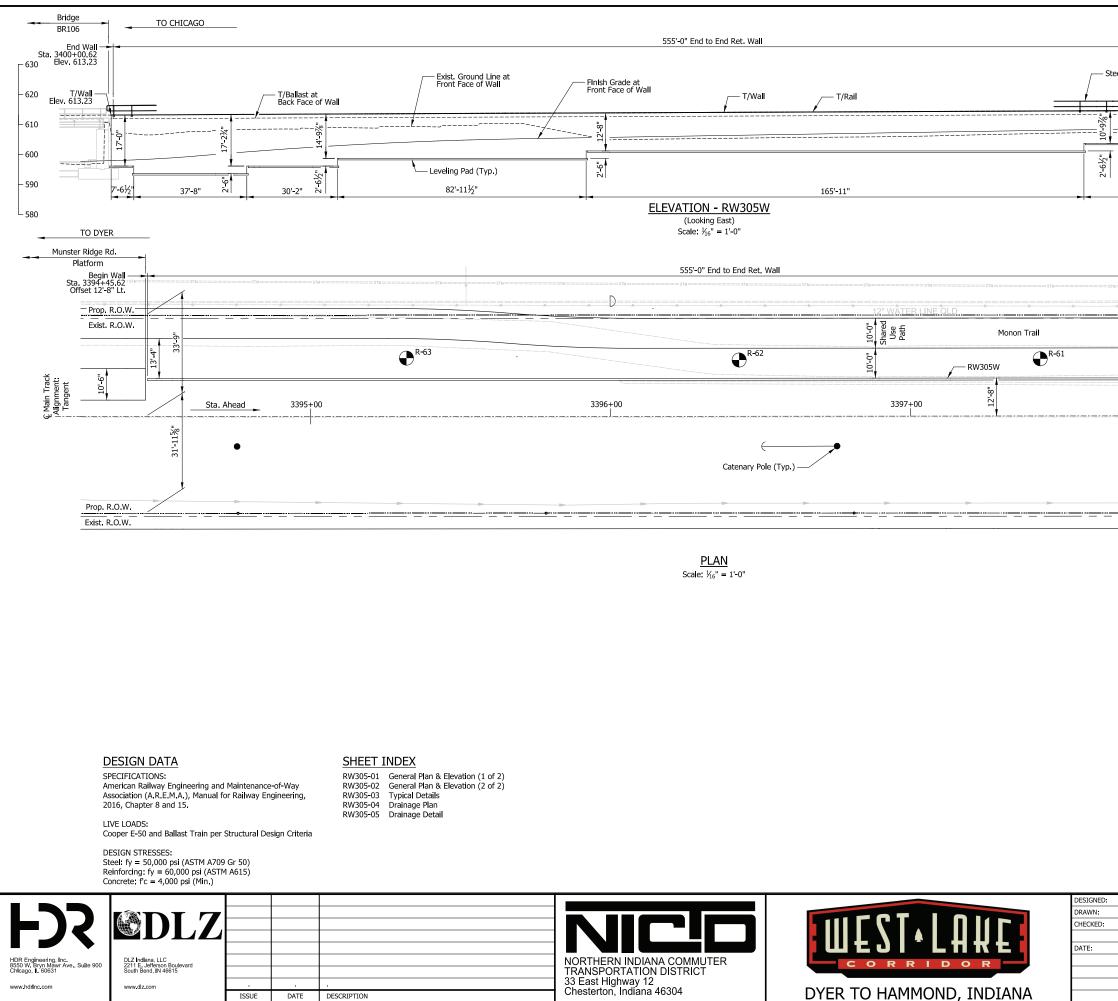


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E Foundation						
1'-0"						
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	PRE	LIMINARY PLANS				
		OR CONSTRUCTION	SERIES			
			RW304-03 OF	RW304-05		
LOCH-EJG LOCH-DJG		NICTD - WEST LAKE CORRIDOR - M SINGLE TRAC				
LOCH-JLH						
		RETAINING WALLS	304W & 30	4E		
07/21/17	TYPICAL DETAILS					
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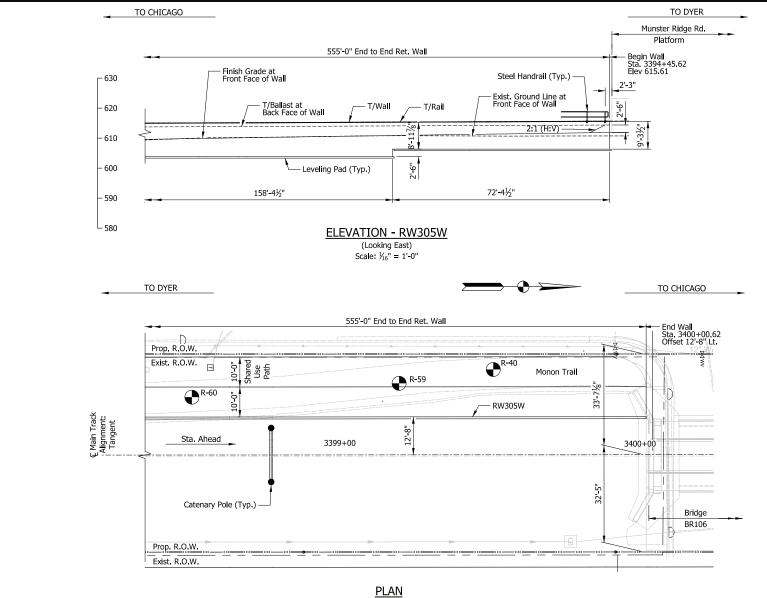








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				TO CHICAGO	
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	3398+00 ⊥		>		
					
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	NOTES				
		track geometry, see Tra clarity Catenary Poles ar		rastructure	
		shown in elevation views d Verify Limits and Depth			Bridge.
		·	-		
		LIMINARY P			
	<u> </u>			SERIES	
	INOT F	OR CONSTRU		RW305-01	OF RW305-05
DLZ-JFM DLZ-JLM		NICTD - WEST LA	KE CORRIDOR - M SINGLE TRAC	P WL 61.38 TO WL 69.3 Ж	18
DLZ-CAG				VALL 305	
07/21/17					_
	GE	NERAL PLA	N & ELE	VATION ((1 OF 2)
	FILENAME	SHT_WL_ST_RW305_Ge		SHEET	
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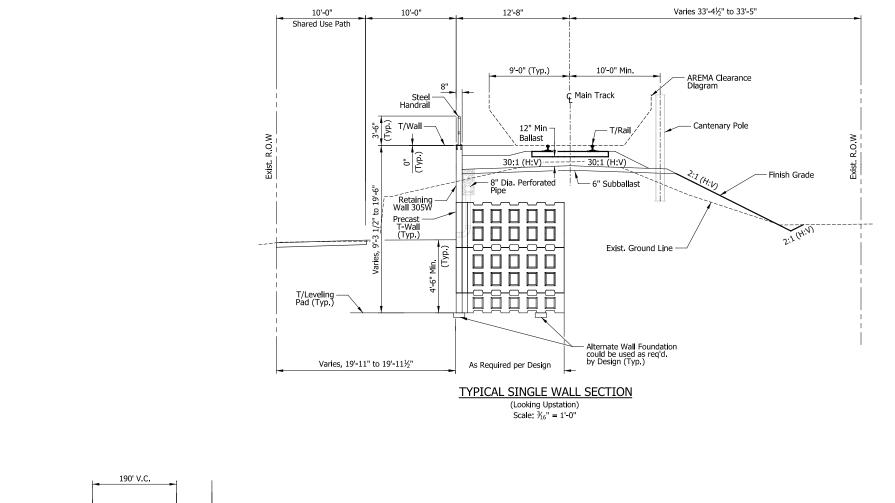


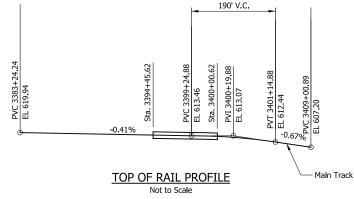
Scale: $\frac{1}{16''} = 1'-0''$



NOTES

- For track geometry, see Track Plans.
 For clarity Catenary Poles and associated infrastructure not shown in elevation views. For details, see System Plans.
 Field Verify Limits and Depth of Existing Footing at Broadmoor Bridge.





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DATE

PLOT

	©DLZ				
0	DLZ Indiana, LLC 2211 E. Jefferson Boulevard South Bend, IN 46615				NORTHERN INE TRANSPORTAT
	www.dlz.com	ISSUE	DATE	- DESCRIPTION	33 East Highway Chesterton, India



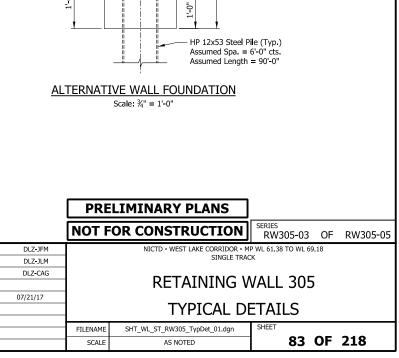


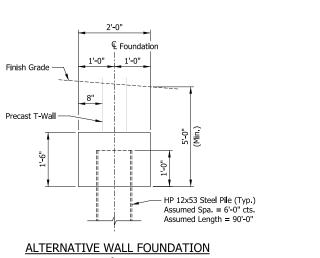
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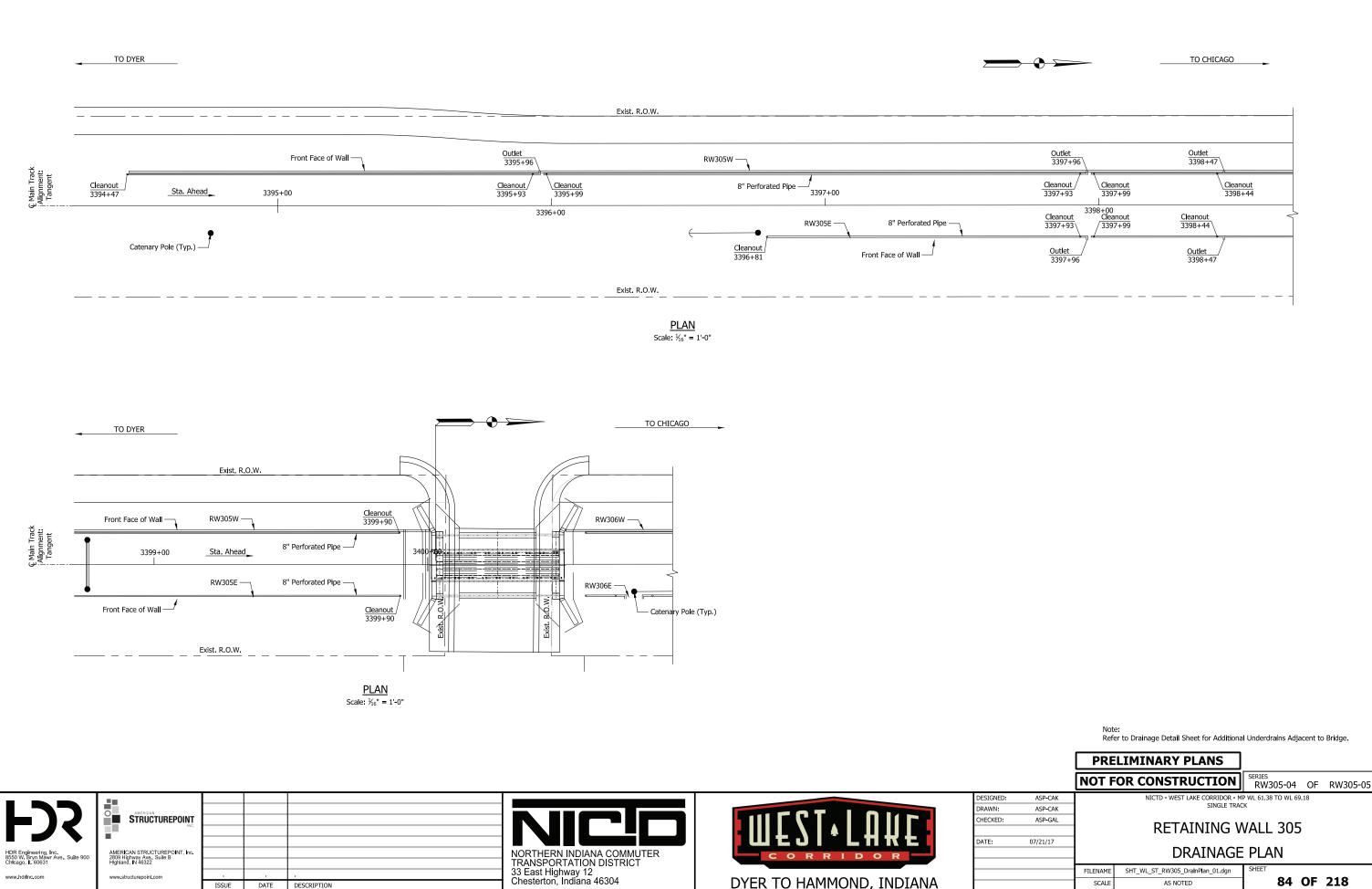
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DATE:

HECKED:



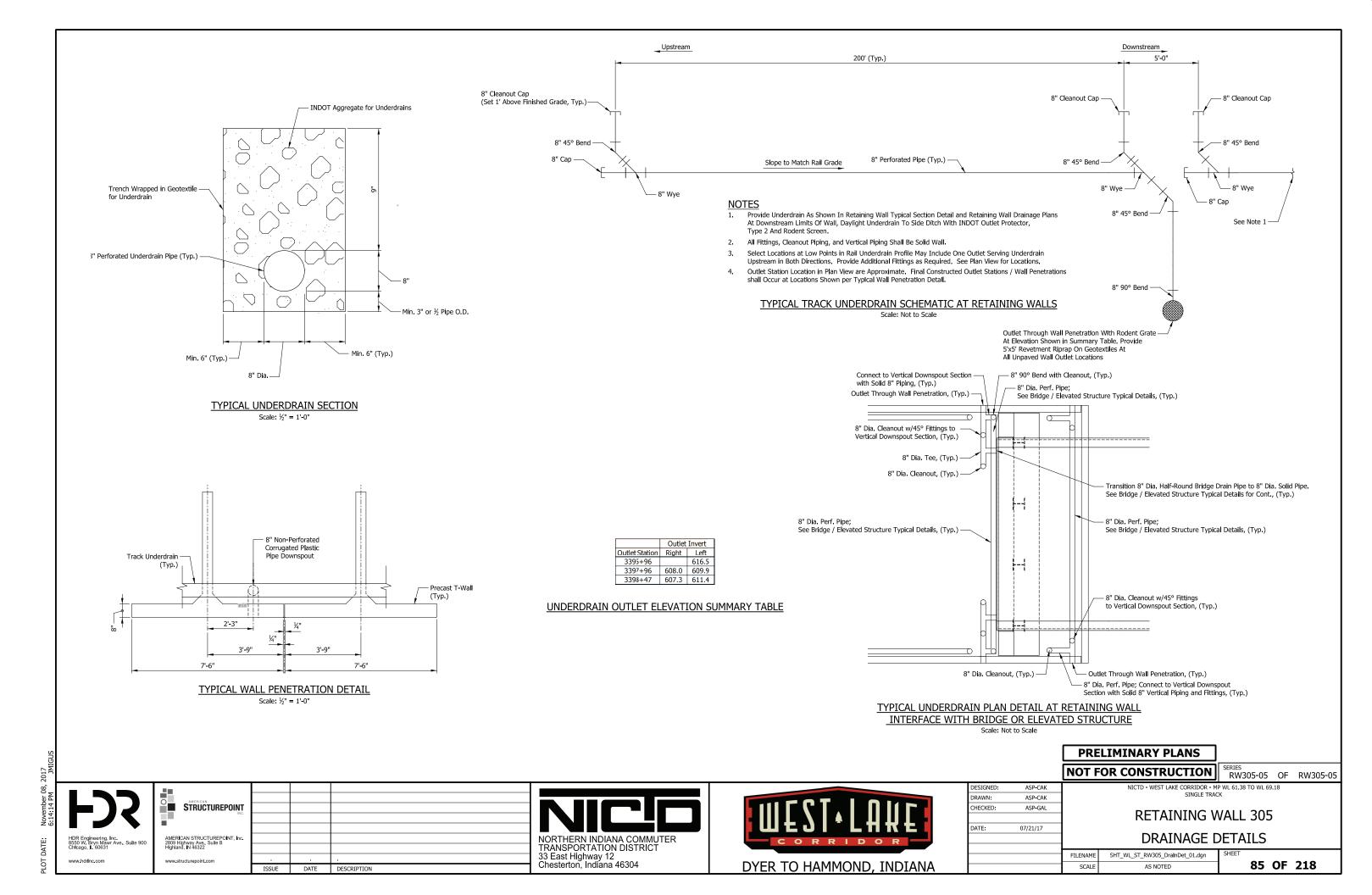


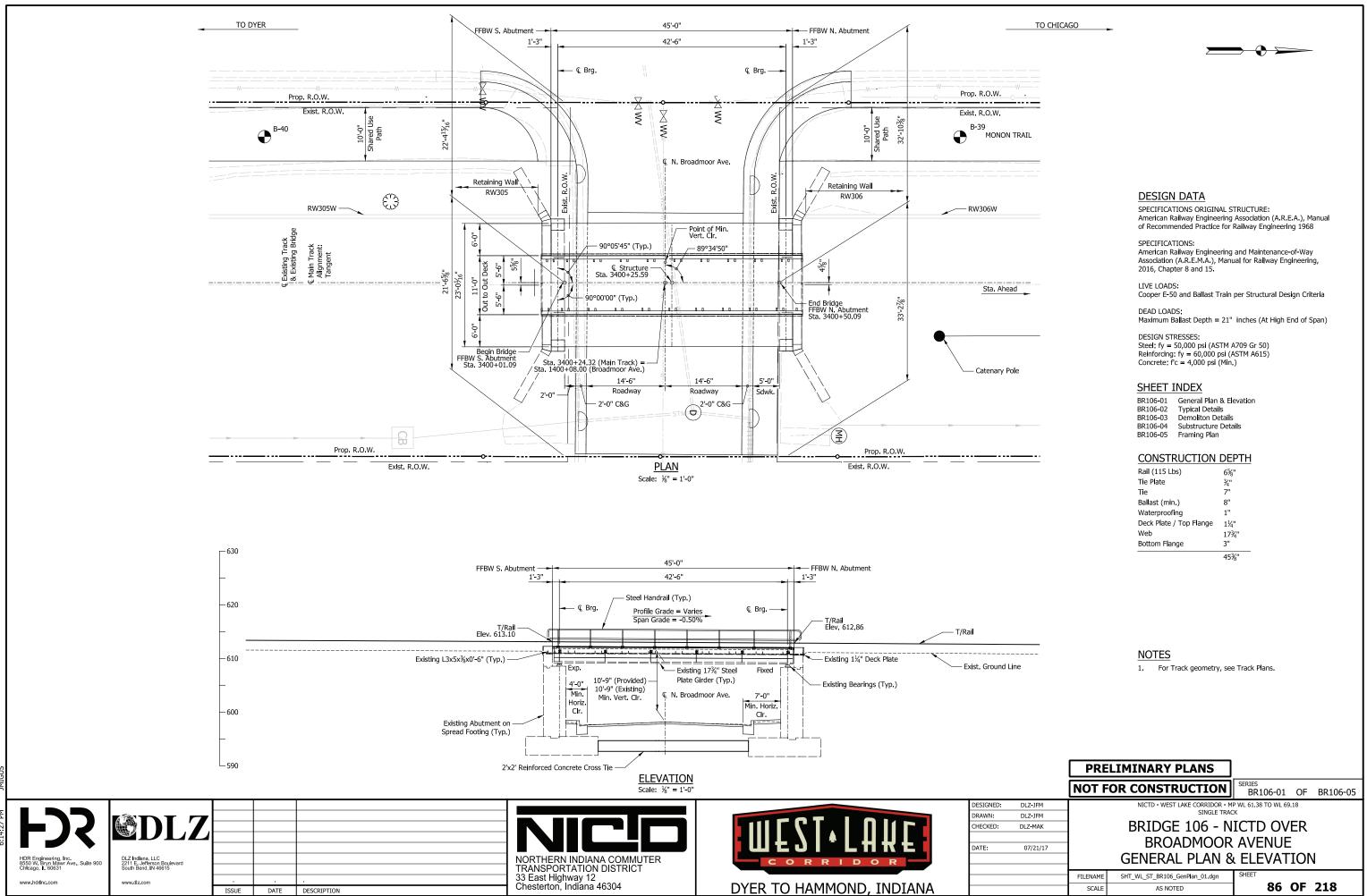


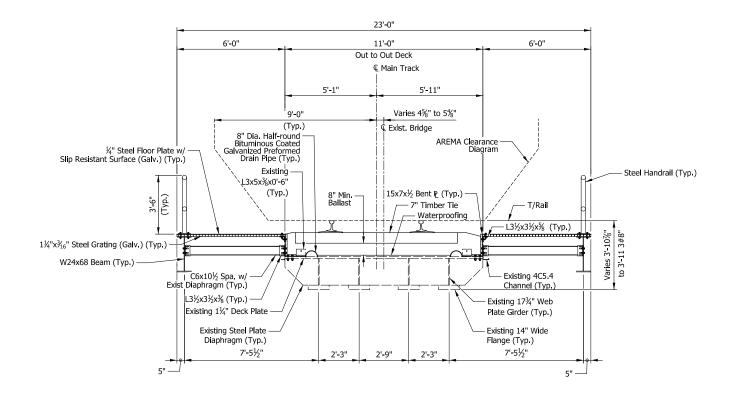
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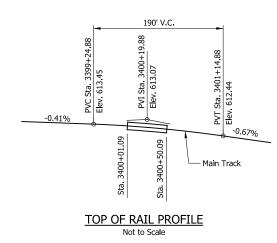
	NOT F	OR CONSTRUCTION	RW305-04	OF	RW305-05			
ASP-CAK		NICTD - WEST LAKE CORRIDOR - M		.18				
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ASP-GAL								
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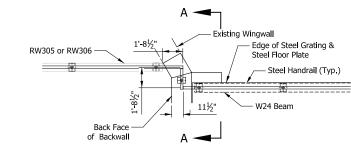






TYPICAL SECTION Scale: ¾" = 1'-0"



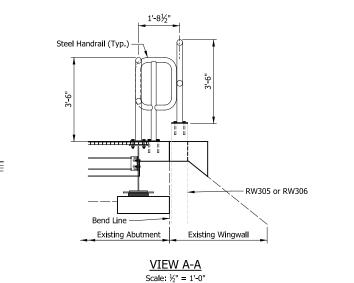


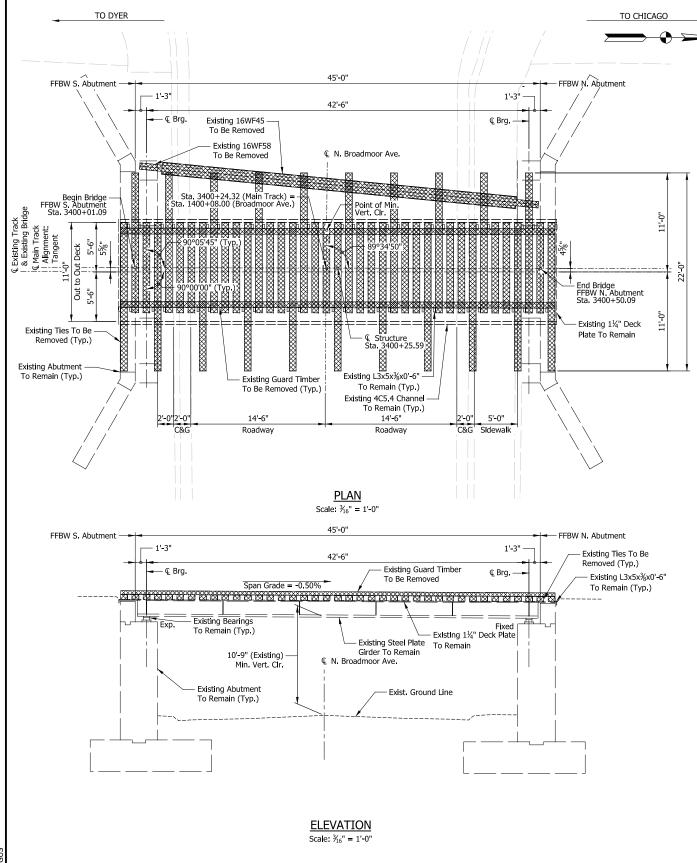
HANDRAIL LAYOUT AT BRIDGE CORNERS Scale: ½" = 1'-0"

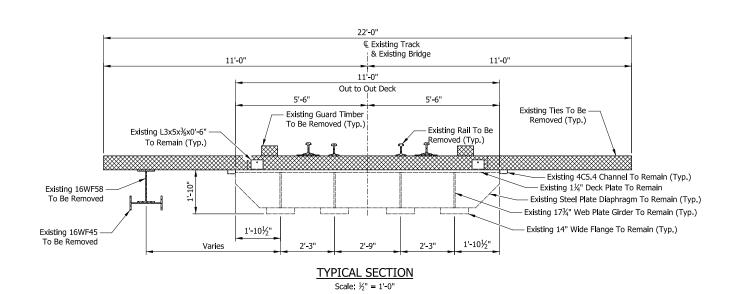


	PRE	LIMINARY PLANS					
	NOT F	OR CONSTRUCTION	SERIES BR106-02	OF	BR106-05		
DLZ-JFM		NICTD - WEST LAKE CORRIDOR - M		.18			
DLZ-JFM							
DLZ-MAK		BRIDGE 106 - N	ICID OV	ER			
		BROADMOOR	Δ\/FNI IF				
07/21/17							
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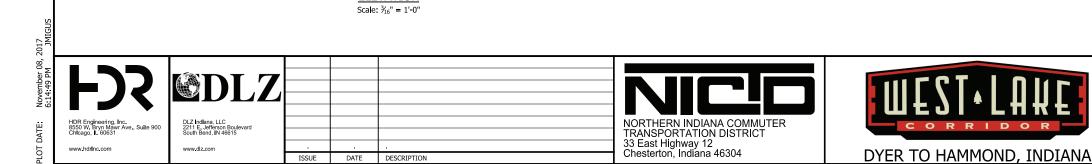




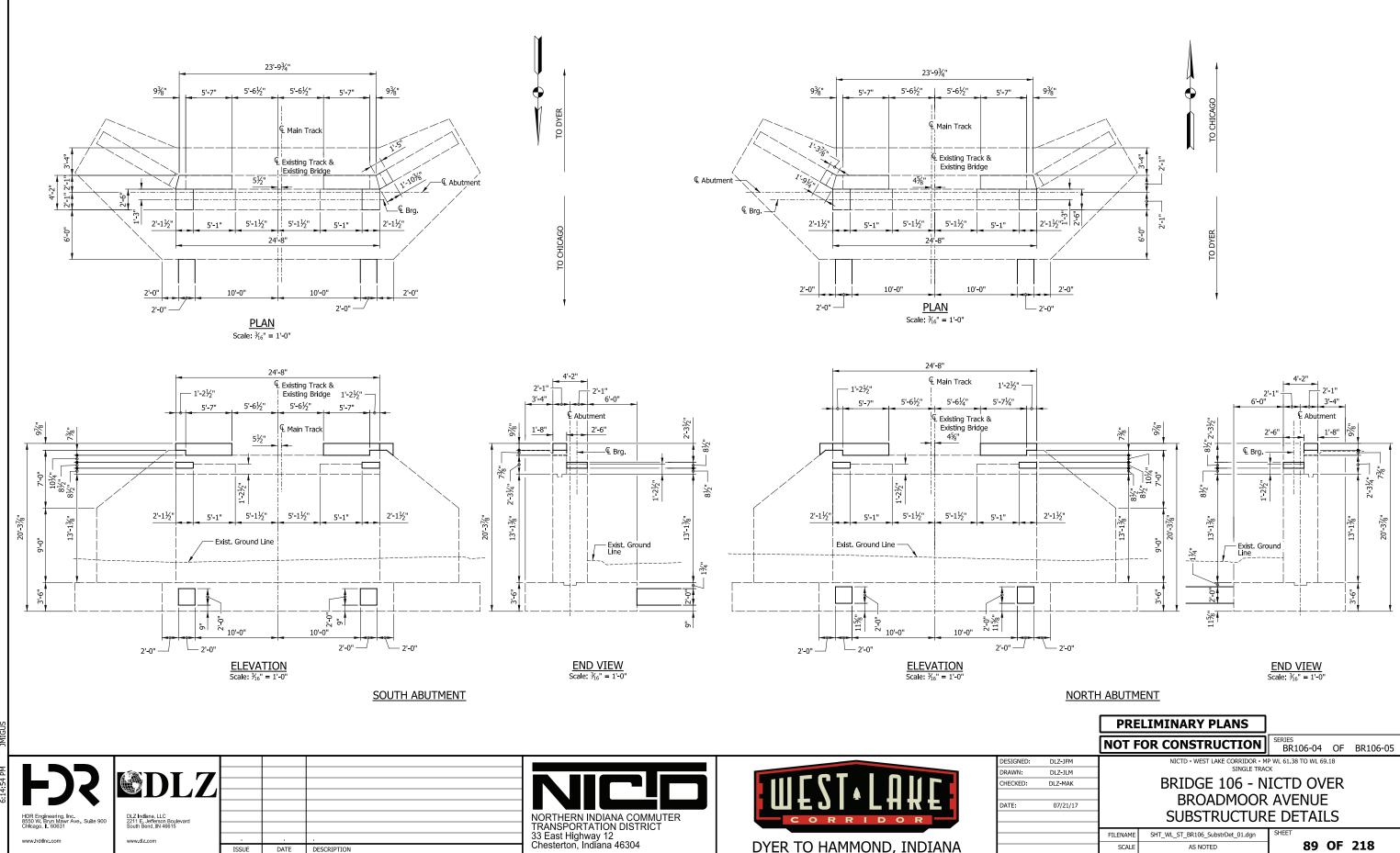
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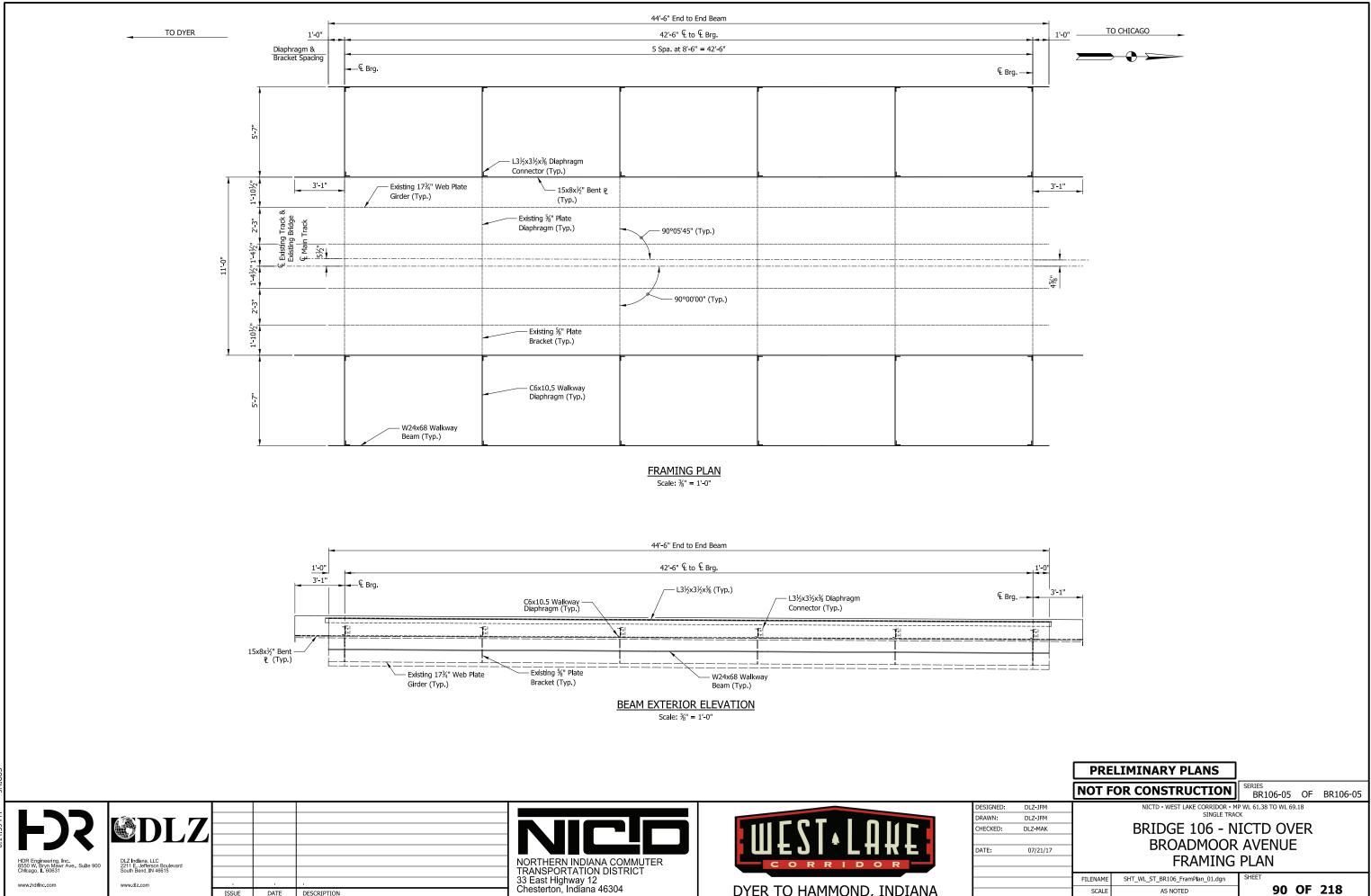
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	<u>NO</u>	TES: Portion of existing structure to be r	emoved.				
	PRE	LIMINARY PLANS					
	NOT F	OR CONSTRUCTION	SERIES BR106	5-03	OF	BR106-05	
DLZ-JFM		NICTD - WEST LAKE CORRIDOR - M SINGLE TRA		0 WL 69	9.18		
DLZ-JFM							
DLZ-MAK	BRIDGE 106 - NICTD OVER						
	BROADMOOR AVENUE						
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	DEMOLITION DETAILS						
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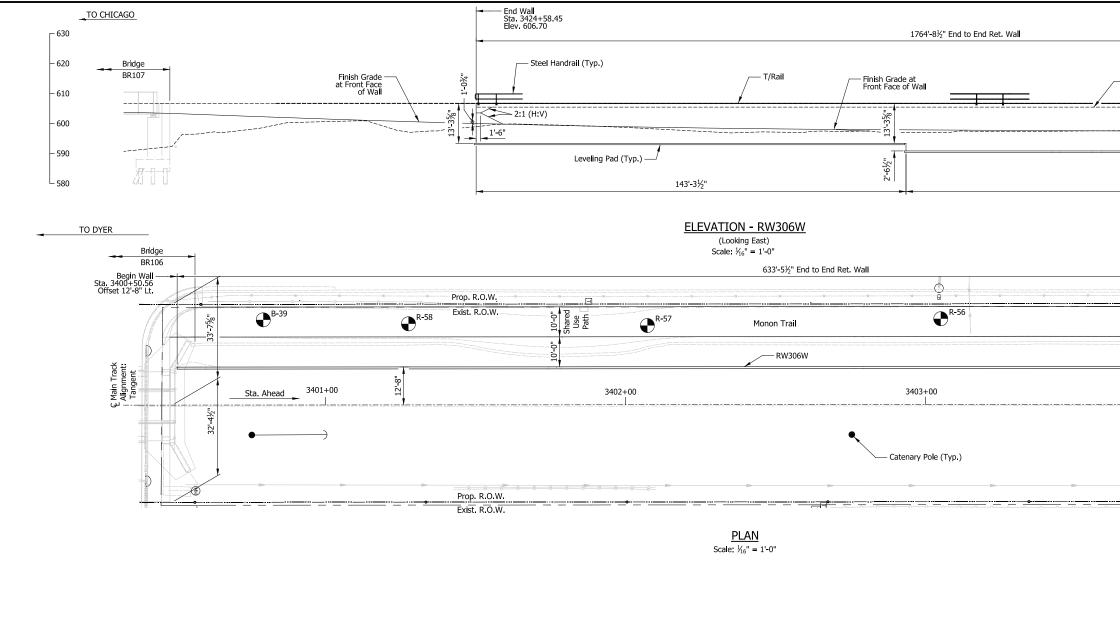




ISSUE

DATE DESCRIPTION

DYER TO HAMMOND, INDIANA



DESIGN DATA SHEET INDEX SPECIFICATIONS: American Railway Engineering and Maintenance-of-Way Association (A.R.E.M.A.), Manual for Railway Engineering, 2016, Chapter 8 and 15. LIVE LOADS: Cooper E-50 and Ballast Train per Structural Design Criteria DESIGN STRESSES: Steel: fy = 50,000 psi (ASTM A709 Gr 50) Reinforcing: fy = 60,000 psi (ASTM A615) Concrete: f'c = 4,000 psi (Min.) RW306-09 Drainage Plan (1 of 2) RW306-10 Drainage Plan (2 of 2) RW306-11 Drainage Detail

DATE

РГОТ

RW306-01 General Plan & Elevation (1 of 6) RW306-02 General Plan & Elevation (2 of 6) RW306-03 General Plan & Elevation (3 of 6) RW306-04 General Plan & Elevation (4 of 6) RW306-05 General Plan & Elevation (5 of 6)
 RW306-06
 General Plan & Elevation (6 of 6)

 RW306-07
 Typical Details (1 of 2)

 RW306-08
 Typical Details (2 of 2) - Drawing Not Used

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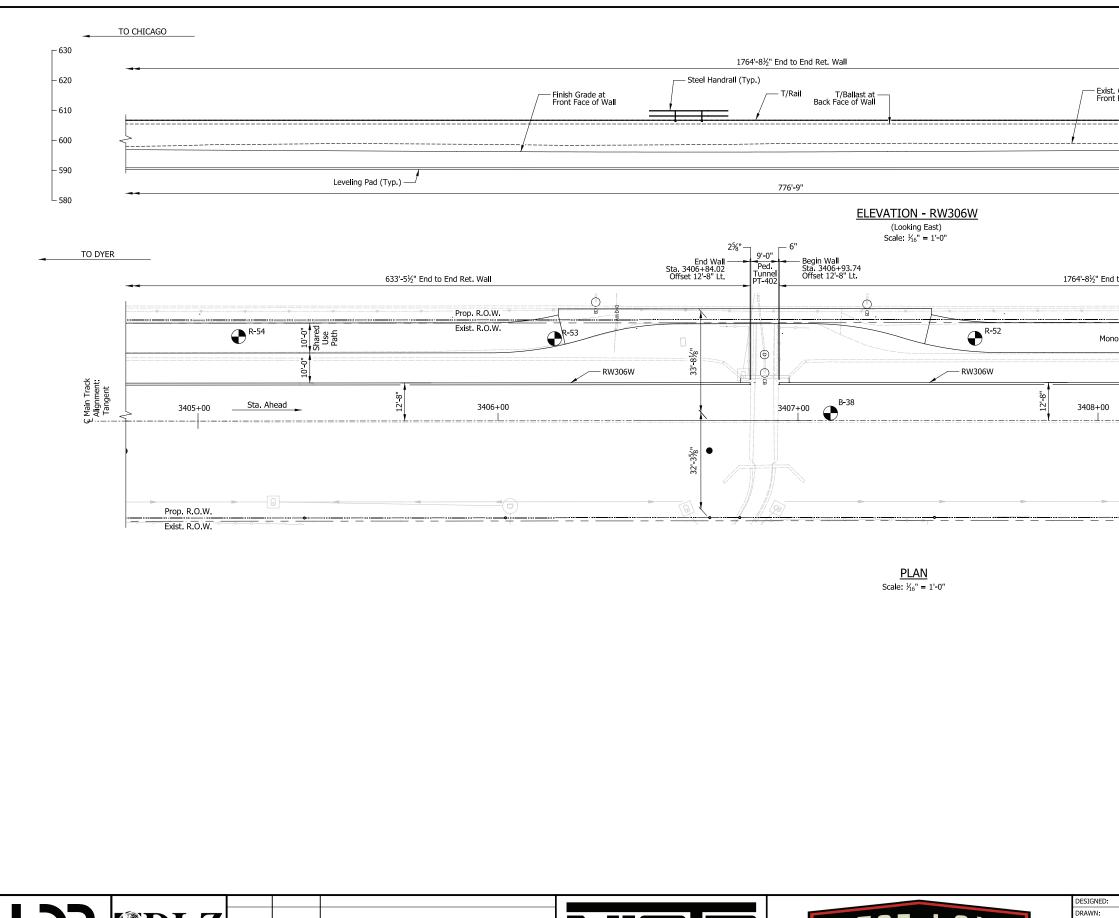
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	NOTES 1. For track geometry, see Track Plans. 2. For clarity Catenary Poles and associated infrastructure not shown in elevation views. For details, see System Plans. 3. Field Verify Limits and Depth of Existing Footing at Broadmoor Bridge	
	PRELIMINARY PLANS	
	NOT FOR CONSTRUCTION	V306-1:
DLZ-JFM DLZ-JLM	NICTD - WEST LAKE CORRIDOR - MP WL 61.38 TO WL 69.18 SINGLE TRACK	
DLZ-CAG 07/21/17	RETAINING WALLS 306W & 306E	
	GENERAL PLAN & ELEVATION (1 OF	6)

91 OF 218



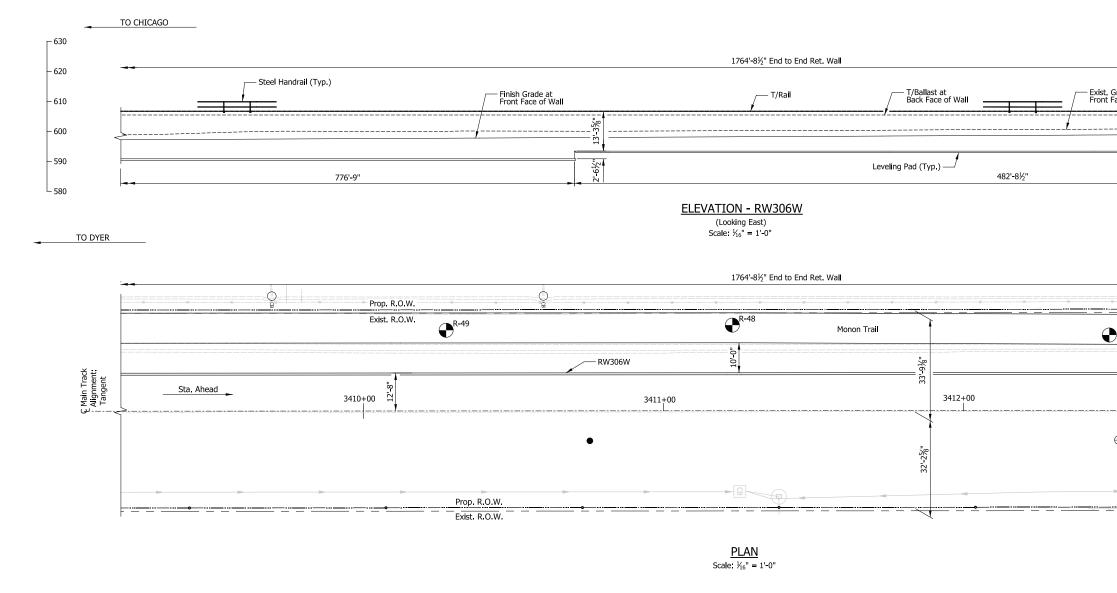
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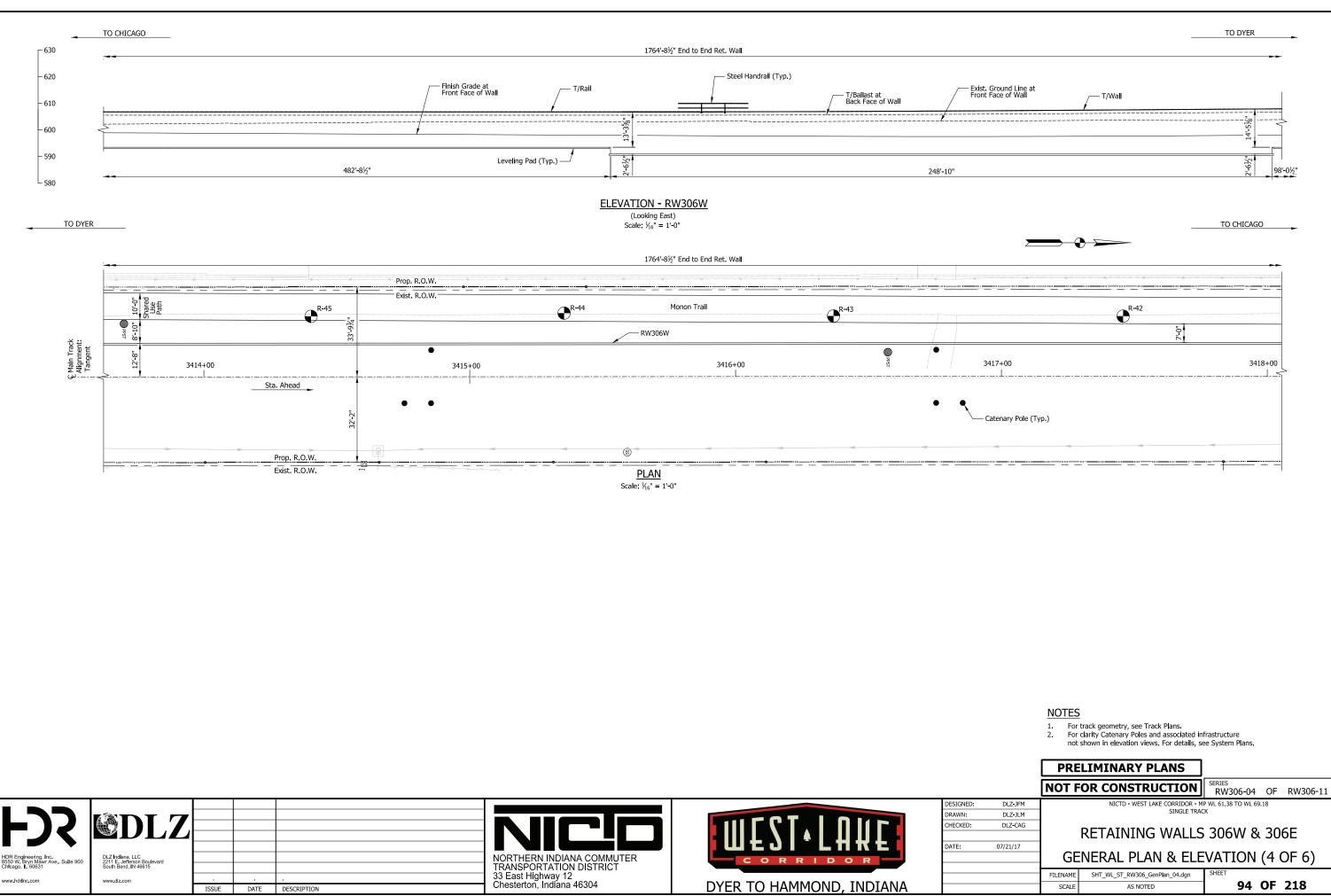
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	not shown in elevation views. For details, see \$ 3. Field Verify Limits and Depth of Existing Footin	System Plans.
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DLZ-JLM DLZ-CAG		
07/21/17	RETAINING WALLS	JUDW & JUDE
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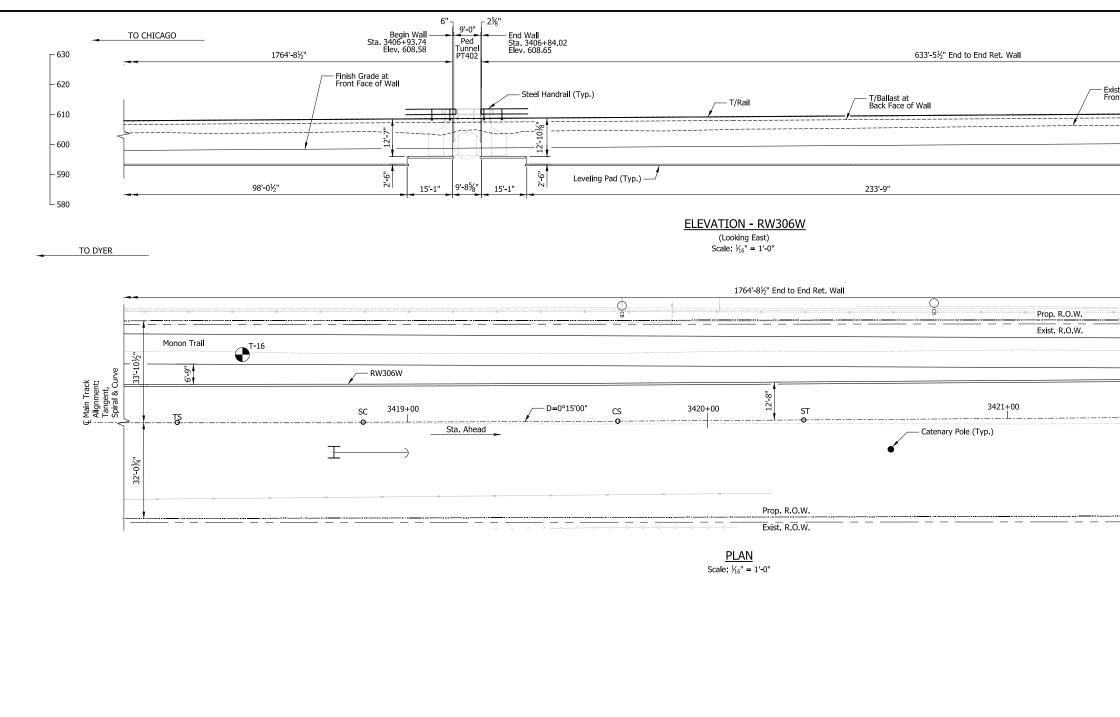
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	PRE		PLANS			
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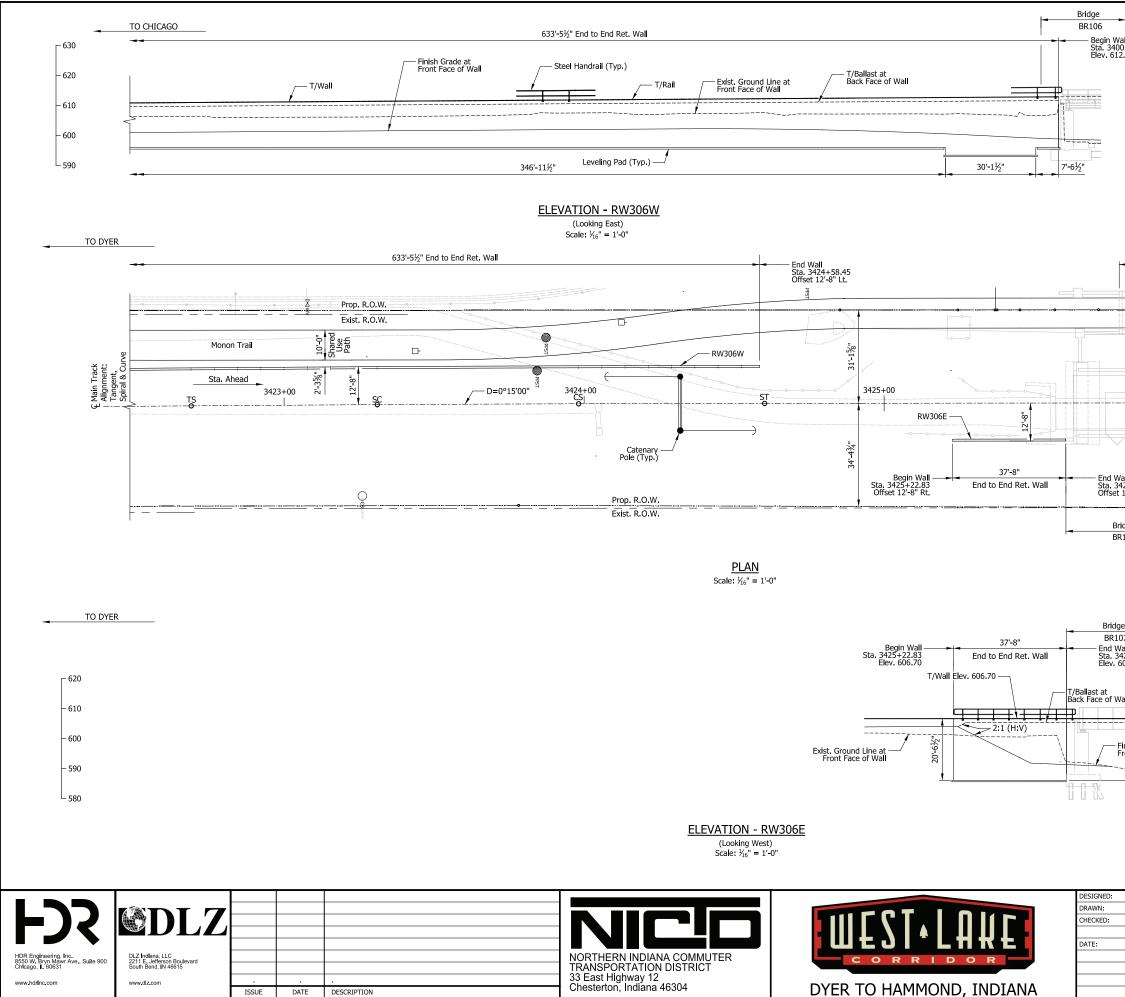
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st. Ground Line at nt Face of Wall			F	T/Wa <b>l</b> l			
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			ry, see Track Plans.	od infr			
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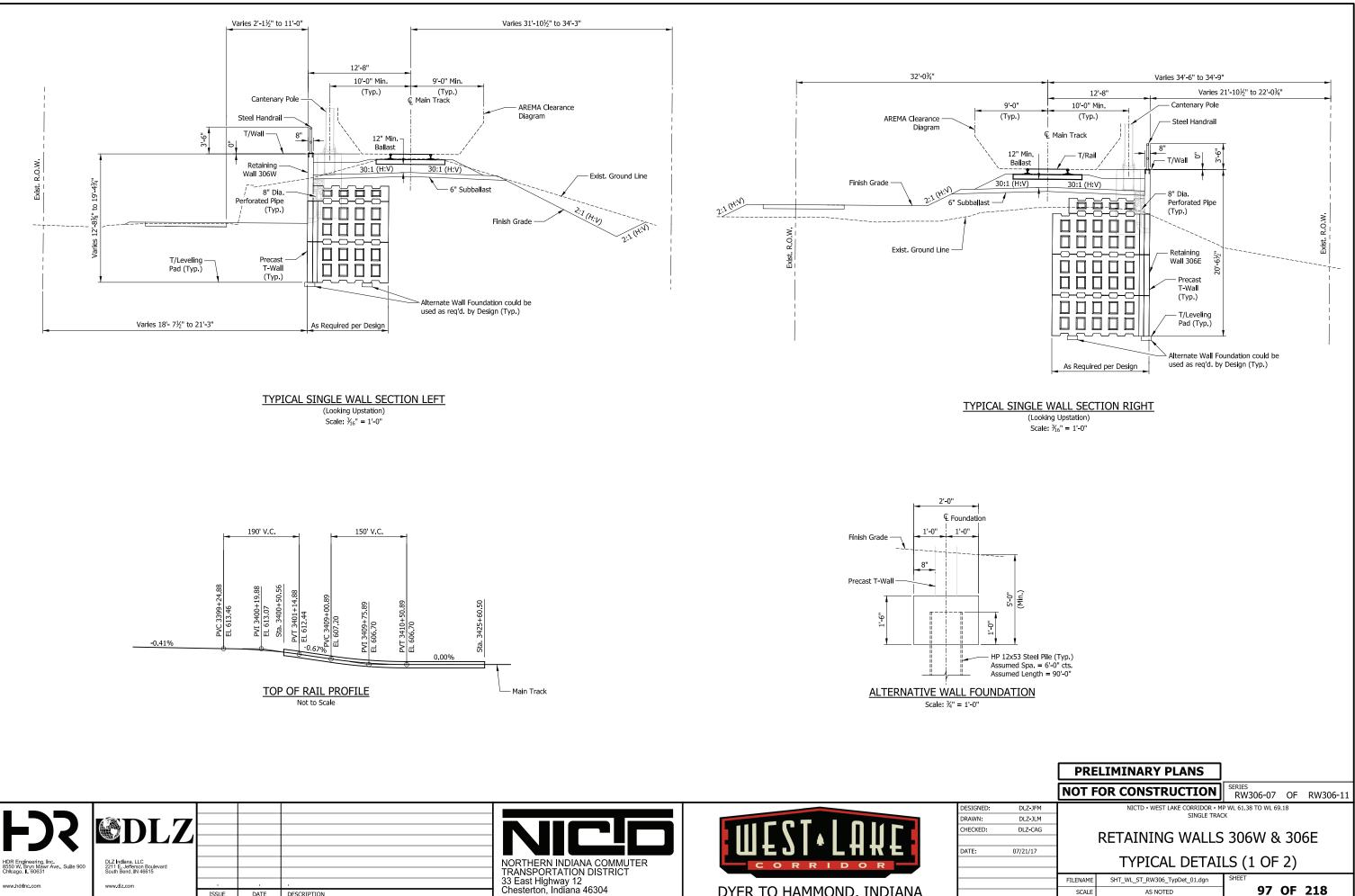
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	TO CHICAGO			
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	<u>NOTES</u>			
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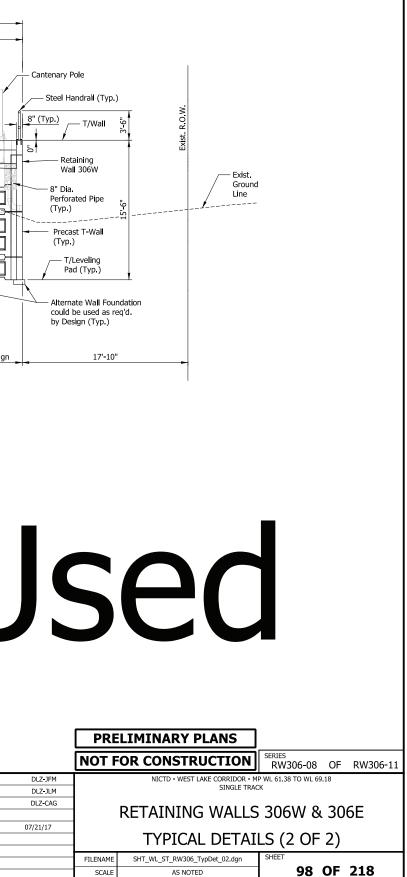
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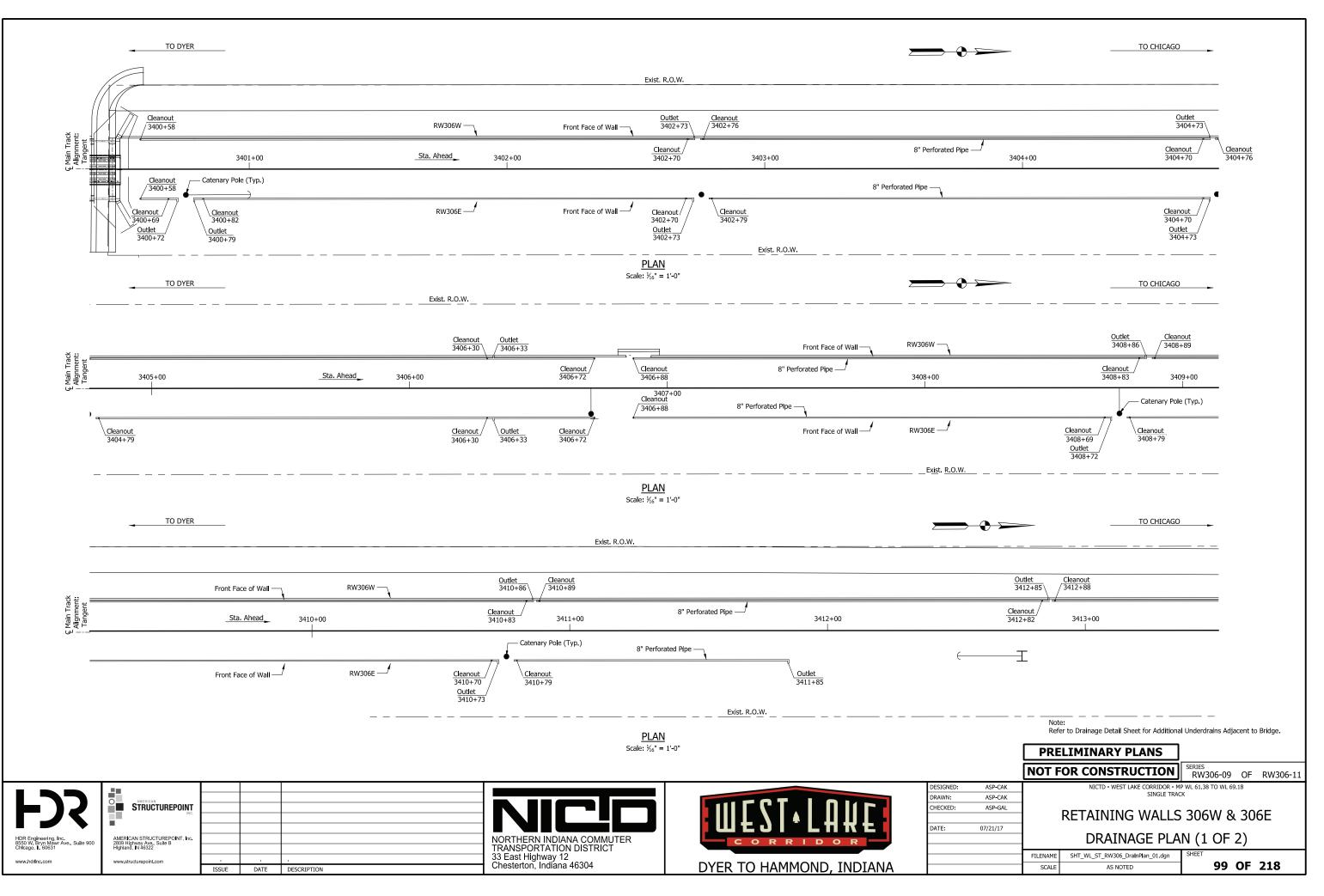




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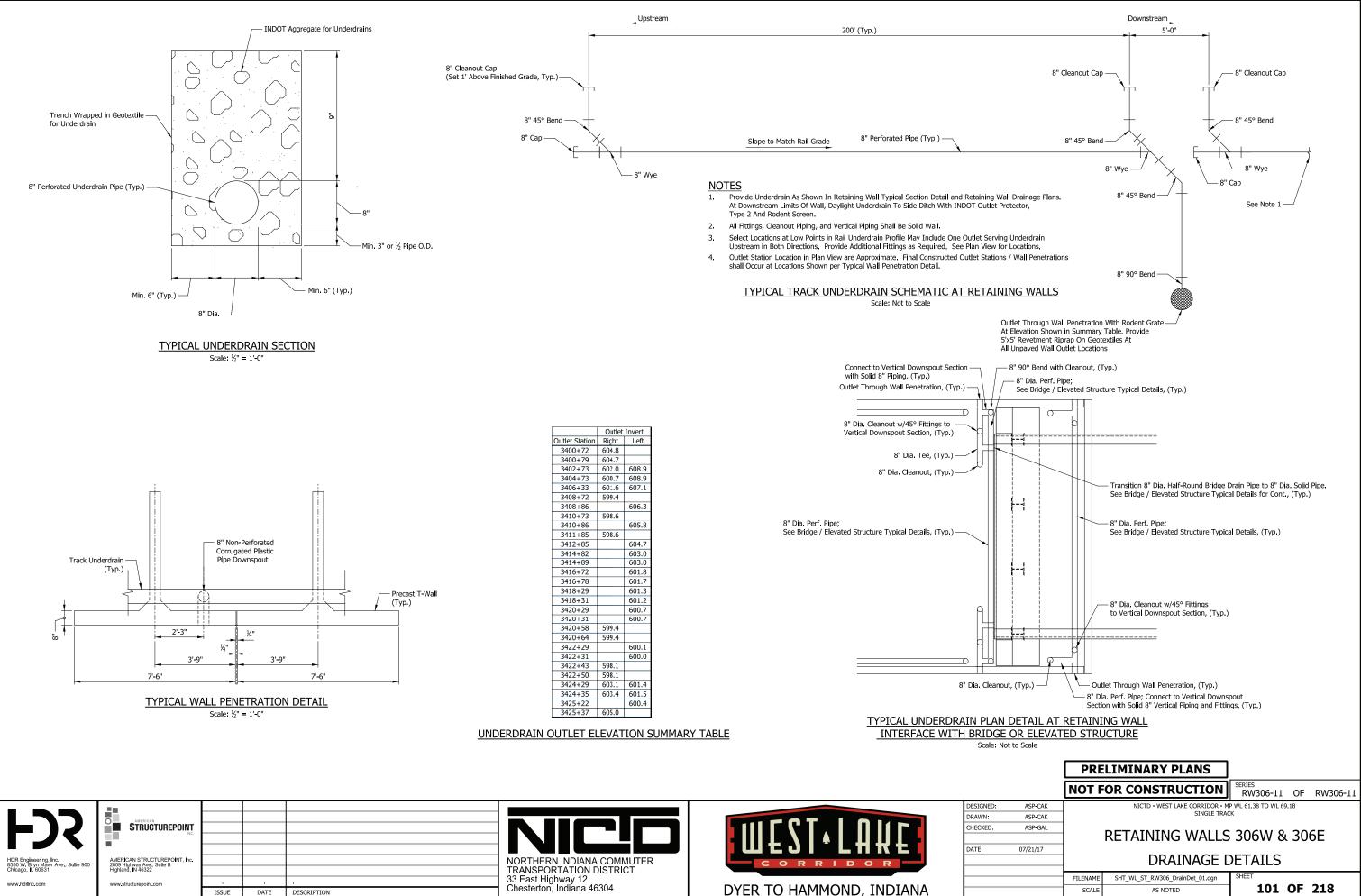
Finish Grade Finish Grade Precast T-Wall Precast T-Wall P	Exist. R.O.W.	17'-2%" Bridge BR107A 10'-0" Shared Use Path EOP Shared Use Path Finish Grade at Rear Face T/Wall Finish Grade at at Front Face T/Wall	n 12" Min. N Ballast	o Out Wall
As Required per Design SECTION THRU END BENT Scale: $\frac{1}{6}$ " = 1'-0"	ing		ELEVATION RET. (Looking Down Scale: 3/16" =	station) 1'-0''
HDR Engineering, Inc.,       DLZ Indiana, LLC         SSO WE, Bryn, Suite 900       DLZ Indiana, LLC         MWW.hdrInc.com       DLZ Indiana, LLC         WWW.hdrInc.com       DLZ Indiana, LLC         SUB Bend, IN 46615       DLZ         WWW.hdrInc.com       DLZ Indiana, LLC         SUB Bend, IN 46615       DLZ         SUB DATE       DESCRIPTION	NORTHERN INDIANA COMMUTER TRANSPORTATION DISTRICT 33 East Highway 12 Chesterton, Indiana 46304		MMOND, INDIAN	DESIGNED: D DRAWN: D CHECKED: DU DATE: 07/2



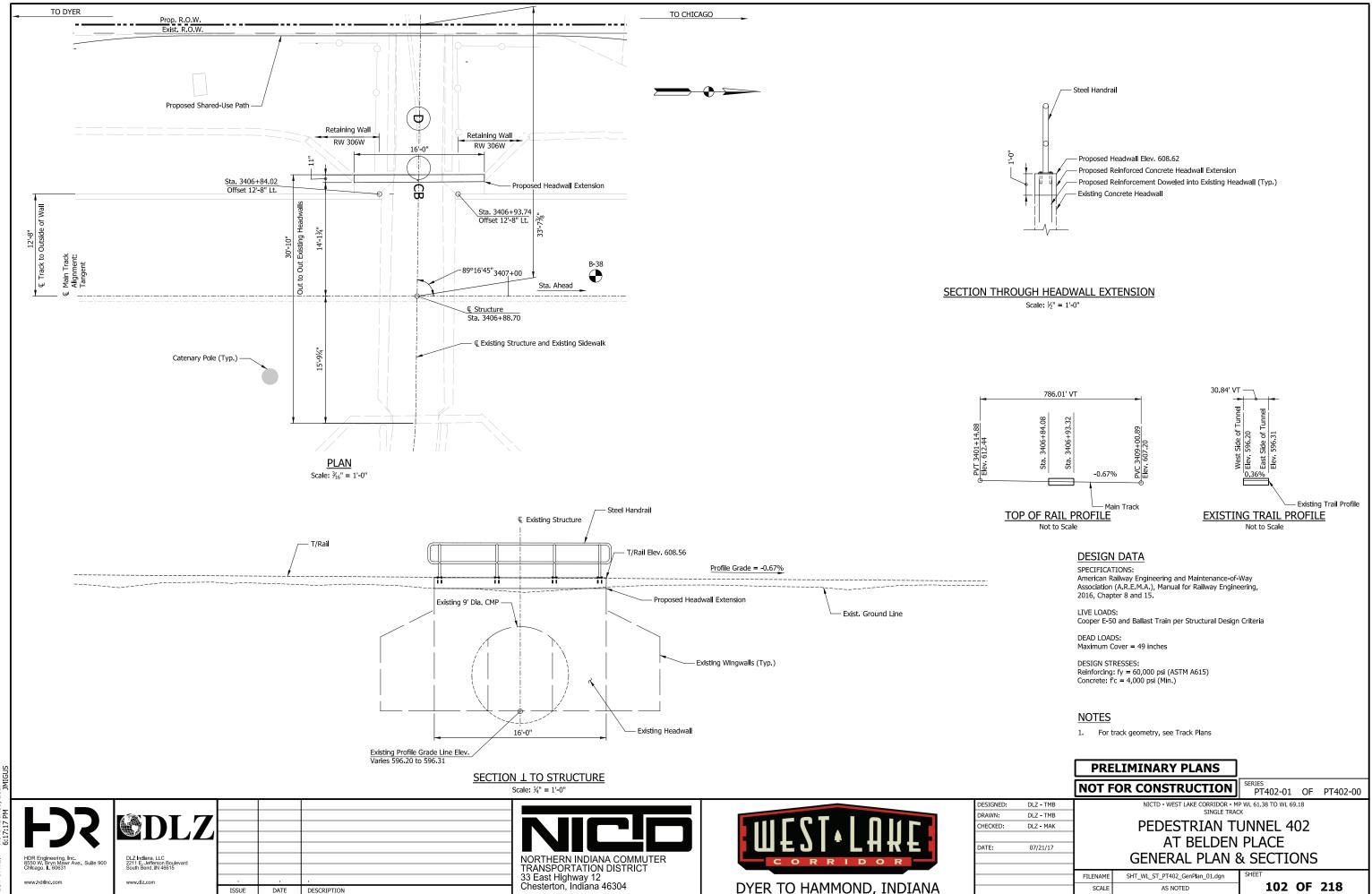


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-		<u> </u>				Exist. <u>R.C</u>	<u></u>				
-				Outlet /	Outlet	RW306W —	Front Fac	e of Wall —	<u>Outlet</u> 3416+72	Outlet 2\ / 3416+78	
=			Sta. Ahead	<u>Cleanout</u> 3414+79	Cleanout	8"	Perforated Pipe		Cleanout	Cleanout	
-	34.	14+00 		3414+79	3414+92			3416+00 	3416+69	3416+81 3417+00	
				••	3413+00					• •	
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-	$\frac{\text{Outlet}}{3418+29} \setminus$					Front Face of Wall	Outlet3420+29	Outlet / 3420+31			Cleanout 3422+26 \
-	Cleanout 3418+26	ıtlet 18+31		RW306V	V — 1						5122720
_	<u>Cleanout</u> 3418+00 3418+34	<u>Cleanout</u> 3418+52	Cleanout           3418+70         34	8" Perforat 419+00	ed Pipe —	Sta. Ahead	<u>Cleanout</u> 3420+00 3420+26	Cleanout 3420+34	3421+00		3422+00
-				Cleanout / 3419+02		8" Perforated Pipe —		<u>Cleanout</u> 3420+55	<u>Cleanout</u> / 3420+67		
			<u> </u>	-) [			· · · · · · · · · · · · · · · · · · ·			RW306E —	
						Front Face	e of Wall —	Outlet 3420+58	Outlet 3420+64		
-					<u>-</u>	xist. R.O.W.	— — — — — —	Catenary Pole (Typ.) —	I		
							PLAN Scale: ½6" = 1'-0"			TO CHICAGO	
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Outlet 3422+29	Outlet / 3422+31	Front Face of Wall —	RW	/306W —		Outlet	Outlet / 3424+35		Cleanout Outlet 3425+02 3425+22	Cleanout 3425+60	
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	422+40 422+53		8" Perforate	d Pipe		3424+26 Outlet	3424+38	3424+65	8" Perforated Pipe	3425+60	
<u>c</u>	Outlet	RW306E		Front Face of Wall		Gatenary Pole (Typ.)	Outlet 3424+35		Outlet		
3	3422+43 3422+50					Exist. R.O.W.	3424+35		3425+37		
-							<u>PLAN</u>		Front Face of Wall –		Note: Refer to Drainage Detail Sheet for Additional Underdrains Adjacent to Bridge.
S							Scale: $\frac{1}{16}$ " = 1'-0"				PRELIMINARY PLANS
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	INC.							ШΕ71	* L H K Ł	DATE: 07/21/17	RETAINING WALLS 306W & 306E
HDR Engineering, Inc. 8550 W. Bryn Mawr Ave., Suite 900 Chicago, IL 60631 www.hdrinc.com	AMERICAN STRUCTUREPOINT, Inc. 2809 Highway Ave., Suite B Highland, IN 46322 www.structurepoint.com				NO TR/ 33 I	RTHERN INDIANA COMMUTI ANSPORTATION DISTRICT East Highway 12 esterton, Indiana 46304	ER		RIDOR		DRAINAGE PLAN (2 OF 2)           FILENAME         SHT_WL_ST_RW306_DraInPlan_02.dgn
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